

## Isle of Wight Council

## Isle of Wight Junction Assessment and Design

Junction Feasibility Study - Binstead Road / Pellhurst Road

## Document Information

| Prepared for | Isle of Wight Council |
| :--- | :--- |
| Project Name | Isle of Wight Junction Assessment and Design |
| File Reference | Feasibilty Study Report - Binstead Road - Pellhurst Road (Jct 8).docx |
| Project Number | A090129-99 |
| Publication Date | February 2018 |

## Contact Information

## WYG Environment Planning Transport Ltd

$11^{\text {th }}$ Floor, 1 Angel Court

London
United Kingdom
EC2R 7HJ
Registered in England \& Wales Number 3050297
Registered office: Arndale Court, Headingley, Leeds, LS6 2UJ
+44 (0)20 72507500
london@wyg.com
www.wyg.com

## Document Control

| Version | Date | Prepared by | Reviewed by | Approved by | Approver Signature |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| D1 | 01.02 .2018 | JS | GS | NW |  |
| Description | Draft for client review |  |  |  |  |

```
Description
```

```
Description
```

```
Description
```

Description

Description

## Limitations

© WYG. Copyright in the whole and every part of this document belongs to WYG and may not be used, sold, transferred, copied or reproduced in whole or in part in any manner or form or in or on any media to any person other than by agreement with WYG. This document is produced by WYG solely for the benefit and use by the client in accordance with the terms of the engagement. WYG does not and shall not assume any responsibility or liability whatsoever to any third party arising out of any use or reliance by any third party on the content of this document.

## Contents

1 Introduction ..... 1
General ..... 1
Site Location \& Background ..... 1
Scope/Purpose of Study ..... 5
Report Structure ..... 5
2 Existing Conditions ..... 6
General ..... 6
Study Area/Junction Background ..... 6
Data Collection ..... 7
Base Traffic Flows ..... 8
Existing Traffic Issues ..... 9
Local Highway Network ..... 10
3 Modelling Methodology ..... 11
Introduction ..... 11
Explanation of Results ..... 11
Base Year Modelling ..... 11
Proposed Scheme ..... 12
Forecast/Future Year Modelling ..... 13
Outcome / Conclusions ..... 14
4 Summary and Conclusions ..... 16
Summary ..... 16
Conclusions ..... 17
Tables
Table 1.1 List of Junctions ..... 1
Table 3.1 2017 Base Year Assessment: Existing Junction ..... 12
Table $3.2 \quad 2034$ Future Year Assessment: Existing Junction ..... 14
Table $3.3 \quad 2034$ Future Year Assessment: Proposed Junction ..... 14
Figures
Figure 1.1 Feasibility Study Area - Newport Junctions ..... 2
Figure 1.2 Feasibility Study Area - Ryde Junctions ..... 3
Figure $1.3 \quad$ Feasibility Study Area - Sandown \& Shanklin Junctions ..... 4
Figure 1.4 Junction Location Plan ..... 5
Figure 2.1 Junction Location Plan ..... 7
Figure 2.2 AM Traffic Flows (PCUs) ..... 8
Figure $2.3 \quad$ PM Traffic Flows (PCUs) ..... 9
Figure $3.1 \quad$ Proposed Junction Layout ..... 13

## Appendices

## Appendix A 1:200 drawings

Appendix B Modelling Output Results

## 1 Introduction

## General

1.1 WYG have been appointed by the Isle of Wight (IOW) Council to undertake a feasibility study for a series of highway junctions located throughout the island, with a particular focus on the towns of Newport, Ryde, Shanklin and Sandown. The study has been carried out to identify traffic management issues impacting on all road users and develop proposals that will form part of a package of schemes to be progressed as part of the Council's Local Implementation Plan (LIP) process.
1.2 IOW Council has identified 15 highway junctions which currently experience traffic issues such as congestion and queuing. The study considers where the main issues lie in relation to traffic movement, road safety, bus operation, pedestrian and cycle provision, public realm, parking provision and servicing. Each of the 15 highway junctions are to be supplemented by a feasibility study report, with traffic modelling software used to test the various proposals in order to identify a range of measures aimed at improving the behaviour and movement of traffic at each junction.
1.3 The study identified 15 key junctions, which are summarised in Table $\mathbf{1 . 1}$ below:

Table 1.1 List of Junctions

| ID Num | Junction Name | Area | Junction Type |
| :---: | :---: | :---: | :---: |
| 1 | St Mary's Roundabout | Newport | 4 arm Roundabout |
| 2 | Coppins Bridge Gyratory | Newport | Gyratory |
| 3 | Hunnyhill/Hunnycross Way | Newport | Signalised Crossroads |
| 4 | Hunnycross Way/Riverway | Newport | $3 x$ Roundabouts |
| 5 | Medina Way/Coppins Bridge Roundabout | Newport | Gyratory |
| 6 | Queens Road/West Street | Ryde | 5 arm Signalised Jct |
| 7 | Argyll St/West St | Ryde | Signalised Crossroads |
| 8 | Binstead Road/Pellhurst Road | Ryde | 3 arm Signalised Jct |
| 9 | Quarr Hill/Newnham Road | Ryde | 4 arm Roundabout |
| 10 | Marlborough Road/Great Preston Road | Ryde | Signalised Crossroads |
| 11 | High Street/Victoria Avenue, Shanklin | Shanklin | 3 arm Signalised Jct |
| 12 | Newport Road/Industrial Way | Shanklin | 4 arm Roundabout |
| 13 | Newport Road/Sandown Road | Shanklin | 3 arm Signalised Jct |
| 14 | Lake Hill/The Fairway | Shanklin | Triangular 3x Priority Jcts |
| 15 | Morton Common/Perowne Way | Sandown | 3 arm Signalised Jct |

## Site Location \& Background

1.4 The Isle of Wight is an island located in the English Channel, approximately 6 km off the Hampshire coast. It is England's largest and second most populous island. The towns of Newport, Ryde, Shanklin and Sandown comprise the project study area for this feasibility study.
1.5 The town of Newport is the largest town on the IOW and is located in the centre of the island, the town of Ryde is located approximately 10 km to the east of Newport, on the north-eastern coast of the island whilst Shanklin and Sandown are located approximately 9 km to the south of Ryde and 10 km to the south east of Newport.
1.6 The IOW as a whole is characterised by high car ownership levels, with $77.5 \%$ of households on the island owning a car or van, as indicated by the 2011 Census. Thus, the private vehicle remains the most convenient and fastest way to travel around the island.
1.7 Figures 1.1-1.3 presents the locations of all 15 junctions within Newport, Ryde and Shanklin \& Sandown, which comprise the feasibility study area.

Figure 1.1 Feasibility Study Area - Newport Junctions


Source: OpenStreetMap with WYG Annotations, September 2017

Figure $1.2 \quad$ Feasibility Study Area - Ryde Junctions


Source: OpenStreetMap, September 2017

Figure 1.3 Feasibility Study Area - Sandown \& Shanklin Junctions


Source: OpenStreetMap, September 2017
1.8 Each of the 15 junctions are to be addressed within an individual feasibility study report. This report focuses on Junction 8, the highway junction of Binstead Road / Pellhurst Road / Queen's Road which comprises a signalised junction in Ryde.
1.9 Figure $\mathbf{1 . 4}$ presents a site location plan of the junction.

Figure 1.4 Junction Location Plan


Source: Google Satellite Image, August 2017

## Scope/Purpose of Study

1.10 The purpose of the study is to identify through traffic modelling software, where the main issues lie in terms of capacity, congestion and queuing at the junction. As part of the traffic modelling, this is to inform the type of highway improvements and design required at each junction.

## Report Structure

1.11 The remainder of this document is structured as follows:

- Chapter 2: Existing Conditions - summarising the existing conditions at the junction, providing background to the junction, local highway network and detailing the current traffic issues experienced at the junction;
- Chapter 3: Modelling Methodology - setting-out details of tasks undertaken to build traffic models of the study area using specialist software, including results of option testing for the junction of interest;
- Chapter 4: Preferred Scheme - detailing the preferred scheme for highway improvements at the junction and their expected outcome; and
- Chapter 5: Summary and Conclusions - summarising the feasibility study process and outlining the key findings of the assessment.
1.12 All Appendices are included at the end of this report for information.


## 2 Existing Conditions

## General

2.1 This chapter establishes the existing, or 'baseline', highway conditions which currently prevail in the area surrounding the junction. It describes the existing local highway network and any traffic issues present at the junction.
2.2 Baseline studies have been informed by detailed site visits and desk-based research carried out between August and September 2017.
2.3 This report focuses on the ' $T$ ' junction at Queen's Road / Pellhurst Road / Binstead Road in Ryde.

## Study Area/Junction Background

2.4 The Queen's Road / Pellhurst Road / Binstead Road ' $T$ ' junction is located on the south-western edge of Ryde, approximately 1 km to Ryde town centre. The A3054 Binstead Road joins Pellhurst Road with Queen's Road forming the northern arm of the 'T' junction.
2.5 A3054 Binstead Road forms the main route to Fishbourne, Pellhurst Road provides the main access to the southern point of Ryde, allowing further access to numerous villages and small towns via Upton Road, to the north is Ryde Golf Club and Merrydale, to the east is Ryde School and Upper Chine and the town centre, to the south is Ryde Cemetery, Ryde Academy and Ryde Lawn Tennis and Croquet Club and to the west is Greenmount Community Primary School and Binstead. A location plan of the junction is provided in Figure 2.1.

Figure 2.1 Junction Location Plan


Source: Google Image, August 2017

## Data Collection

2.6 Traffic flow surveys were undertaken by MHC Traffic Ltd on Thursday $20^{\text {th }}$ July 2017 to establish the baseline traffic conditions for the local highway network on the IOW. A range of surveys were undertaken including:

- Manual Classified Counts (MCC) for turning flow information at 15 key junctions on the IOW;
- Automatic Traffic Counts (ATCs) were placed at strategic locations on the network allowing the speeds to be obtained at each of these junctions;
- Queue length surveys at stop lines of all 15 junctions; and
- Traffic video surveys at each of the 15 junctions.
2.7 The surveys allowed for the identification of turning movements at all key junctions as well as routing within the IOW. Signal timing data was additionally supplied by the IOW Council for use for the correct modelling of signal timing data.
2.8 The data collected as part of the surveys was used directly for calibrating and validating the base scenario for both the Junctions 9 and LinSig models. Video footage of the surveyed junctions was additionally reviewed to ensure that the base models reflect the on-street road conditions as closely as possible.


## Base Traffic Flows

2.9 This section details the current traffic flows and queuing at the junction, as indicated by the survey data. These are shown in Figures 2.2 and 2.3.

Figure 2.2 AM Traffic Flows (PCUs)

2.10 As shown in Figure 2.2, the highest traffic flows are those travelling northbound in the AM peak (08:0009:00) with 550 PCUs undertaking this movement. The high northbound flows are likely to be associated with those commuting to Newport for work. Southbound flows are also particularly high, with 375 PCUs undertaking this movement.
2.11 In terms of queues, the majority of queuing occurs on Pellhurst Road ( S ), which is largely due to high traffic flows travelling from Argyll Street to Pellhurst Road (northbound), at the Argyll Street junction located further to the south along Pellhurst Road. The maximum observed queue recorded on Pellhurst Road ( S ) was eight vehicles.
2.12 Some queuing also occurs on Queen's Road and Binstead Road, with a maximum observed queue of five vehicles recorded on these arms. Figure $\mathbf{2 . 3}$ below shows the traffic flows for the PM peak (17:0018:00).

Figure 2.3 PM Traffic Flows (PCUs)

2.13 As shown in Figure 2.3, the highest traffic flows are those travelling northbound in the PM peak (17:0018:00), with 700 PCUs undertaking this movement. Southbound flows are also particularly high, with 426 PCUs recorded.
2.14 Again as observed in the AM peak, the majority of queuing also occurs on Pellhurst Road in the PM peak. The maximum observed queue recorded on Pellhurst Road was thirteen vehicles. Queuing also occurs on Binstead Road and Queen's Road, with maximum observed queues of nine and eight vehicles, respectively.

## Existing Traffic Issues

2.15 At present, the junction is known to experience some queuing, of which has been informed by a site visit and traffic video surveys at the junction. It was observed that congestion and queuing is particularly evident on Pellhurst Road.
2.16 As previously noted, there is a significant flow of vehicles travelling northbound and southbound on Pellhurst Road/Binstead Road in both the AM (08:00-09:00) and PM (17:00-18:00) peaks, this is largely due to high traffic flows travelling from Argyll Street to Pellhurst Road (northbound).
2.17 In general, there is minimal queuing on Queen's Road in both the AM (08:00-09:00) and PM (17:0018:00) peaks.

## Local Highway Network

## A3054 Binstead Road

2.18 Binstead Road is a two-way single carriageway road with footways either side of the carriageway and a signalised pedestrian island on the approach to Queen's Road. The road is subject to a 30 mph speed restriction.

## Mayfield Road

2.19 Mayfield Road is a two-way single carriageway road, however, access is permitted only for residents heading westbound onto the road, whilst vehicular access eastbound onto the junction is restricted to buses only. Vehicular access is permitted left and right into Mayfield Road, from Pellhurst/Binstead Road. Footways are provided either side of the carriageway. The road provides access to Greenmount Community Primary School and is subject to a 20 mph speed restriction as a result.

## Queen's Road

2.20 Queen's Road is a two-way single carriageway road with footways either side of the carriageway. Highfield Nursing Home, Ryde Junior School and All Saints church are all situated along Queen's Road.
2.21 There is a mandatory cycle lane approximately 50 m in length that runs up until the stop line at the western end of Queen's Road, leading onto Binstead Road. The road is also subject to a 30 mph speed restriction.

## Pellhurst Road

2.22 Pellhurst Road is a two-way single carriageway road with footways either side of the carriageway. Ryde Academy, Ryde Sports Centre, Ryde Health and Wellbeing Centre can all be accessed via Pellhurst Road. It forms one of the main routes to the south leading to Upton, via Upton Road. The road is subject to a 30 mph speed restriction.

## Utilities Assessment

2.23 A utilities assessment has been carried out at the junction as an indication of which utilities are present within the vicinity of the junction. The following utilities which may be affected by improvements at the junction are listed below:

- Isle of Wight Council
- Openreach - (British Telecommunications)
- Scottish and Southern Electricity - Gas
- Scottish and Southern Electricity - High Voltage
- Scottish and Southern Electricity - Low Voltage
- SGN - Southern Gas Networks
- Southern Water - Sewer
- Southern Water - Water


## 3 Modelling Methodology

## Introduction

3.1 Traffic modelling has been undertaken as part of the feasibility study, identifying how the local highway network on the Isle of Wight currently operates and how it might operate following the proposed improvements to the identified junctions. LinSig v3 is the latest version of JCT's industry-standard software for modelling signalised junctions and urban road networks and has therefore been used to model this junction.
3.2 The modelling has been undertaken for two weekday periods considered to be the network peaks in terms of traffic volumes, with the AM peak between 08:00 and 09:00 and the PM peak between 17:00 and 18:00. These peaks were identified through analysis of traffic count data. Initially, Base Year modelling was using survey data collected in July 2017. Future Year modelling was subsequently carried out in order to test the proposed changes to the network and assess the scale of impact on road traffic.

## Explanation of Results

3.3 Queue lengths at junction approaches are usually expressed in terms of 'Passenger Car Equivalent' (PCE) or 'Passenger Car Unit' (PCU). A standard car typically has a PCE/PCU value of 1.0; larger vehicles, such as goods vehicles, typically have PCE/PCU values greater than 1.0 and smaller vehicles, such as motorcycles, typically have PCE/PCU values less than 1.0.
3.4 The Degree of Saturation (DoS) is a ratio of demand to capacity on each approach to a signalised junction, with a value of $100 \%$ meaning that demand and capacity are equal and no further traffic is able to progress through the junction. Values over $85 \%$ are typically regarded as suffering from traffic congestion, with queues of vehicles beginning to form. The term Practical Reserve Capacity (PRC) is often used to refer to the available spare capacity at a junction. A negative PRC indicates that the junction is over capacity.
3.5 It is noted that a DoS of $90 \%$ or over recorded on an approach to the junction is deemed as approaching capacity and therefore a DoS of under $90 \%$ is considered acceptable.

## Base Year Modelling

3.6 Data was collected as part of the surveys was compared to the base year outputs to match modelled flows and queue patterns to those observed, within acceptable variations. The results for the A3054 Binstead Road / Pellhurst Road / Queen's Road junctions are summarised in Table 3.1, with full output results included in Appendix B.

## LinSig Modelling Results - 2017 Base Year

Table 3.1 2017 Base Year Assessment: Existing Junction

|  | AM peak |  |  | PM peak |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Arm <br> Cycle time 55 secs | Deg Sat (\%) | Mean Max Queue (pcu) |  | Deg Sat (\%) | Mean Max Queue (pcu) |  |
| 1/2+1/1-Binstead Road | 60.7\% | 5.4 | 9.8 | 80.1\% | 9.9 | 16.8 |
| 2/1- Queen's Road | 61.5\% | 4.2 | 30.0 | 78.6\% | 7.3 | 33.9 |
| 3/1 - Pellhurst Road | 41.4\% | 4.2 | 10.3 | 45.3\% | 4.7 | 12.7 |
| PRC | 46.3\% |  |  | 12.3\% |  |  |
| Total Delay (pcu/hr) | 5.19 |  |  | 9.10 |  |  |

3.7 The base year results as shown in Table 3.1, indicate that the existing junction operates within recommended capacity during the AM and PM peaks. For instance, the junction currently records a PRC of $46.3 \%$ in the AM peak and a PRC of $12.3 \%$ in the PM peak. The highest modelled queues were recorded on Binstead Road in the PM peak, which comprised 10 vehicles. Overall, the junction performs well with sufficient spare capacity in the 2017 Base Year, however it is noted that the PM peak exhibits lower reserve capacity in comparison to the AM peak.

## Proposed Scheme

3.8 A proposed scheme has been developed and is to be tested as part of the traffic modelling. This scheme includes relocating the pedestrian crossings on Queen's Road and Pellhurst Road, with one of the central pedestrian islands removed, resulting in a reduction of two pedestrian crossings. There is also to be a realignment of the kerb line between Queen's Road and Binstead Road / Pellhurst Road, on the northern and southern side of the carriageway. Mayfield Road is to be made one-way eastbound only, with traffic only permitted onto the junction. This scheme is primarily focused on improving the crossing facilities and public realm for pedestrians.
3.9 The proposed layout is presented below in Figure 3.1. The 1:200 drawing is included at Appendix A.

Figure 3.1 Proposed Junction Layout


Source: WYG Drawing A090129-99-008, August 2017

## Forecast/Future Year Modelling

3.10 Forecast or Future Year modelling was undertaken for the existing junction and the proposed junction design (as shown in Figure 3.1), to provide a comparison between the two junction designs. The results for the existing and proposed junction design are summarised in Tables 3.2 and 3.3, with full output results included in Appendix B.

LinSig Modelling Results - Future Year
Table 3.2 2034 Future Year Assessment: Existing Junction

| Arm <br> Cycle time 55 secs | AM peak <br> Deg Sat <br> (\%) |  |  | Mean <br> Max <br> Queue <br> (pcu) | Av. <br> Delay <br> Per <br> PCU <br> (s/pcu) | Peg Sat <br> (\%) |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
|  | $92.9 \%$ | 15.8 | 34.4 | $115.0 \%$ | Mean <br> Max <br> Queue <br> (pcu) | Av. <br> Delay <br> Per <br> PCU <br> (s/pcu) |
| 2/1 - Queen's Road | $80.6 \%$ | 6.4 | 43.3 | $116.9 \%$ | 47.3 | 320.6 |
| $3 / 1$ - Pellhurst Road | $54.5 \%$ | 5.7 | 11.6 | $52.8 \%$ | 5.6 | 11.9 |
| PRC | $-3.2 \%$ |  | $-29.8 \%$ |  |  |  |
| Total Delay (pcu/hr) | 13.40 |  | 121.77 |  |  |  |

3.11 The Future Year scenario indicates that the existing junction is expected to exceed maximum capacity in both peak periods, particularly so in the PM peak. Binstead Road suffers the largest reduction in capacity, with its DoS recorded as over $90 \%$ in the AM and over $100 \%$ in the PM. There is also predicted to be a significant worsening in queuing on Binstead Road and Queen's Road. Overall, the PRC is predicted to be $-3.2 \%$ in the AM peak and $-29.8 \%$ in the PM peak.

## LinSig Modelling Results - Proposals

Table 3.32034 Future Year Assessment: Proposed Junction

|  | AM peak |  |  | PM peak |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Arm <br> Cycle time 55 secs | $\begin{gathered} \text { Deg Sat } \\ (\%) \end{gathered}$ | Mean Max Queue (pcu) |  | Deg Sat (\%) | Mean Max Queue (pcu) |  |
| 1/2+1/1 - Binstead Road | 195.6\% | 221.9 | 941.2 | 191.0\% | 266.0 | 917.5 |
| 2/1- Queen's Road | 123.7\% | 37.8 | 411.4 | 194.2\% | 132.0 | 955.1 |
| 3/1- Pellhurst Road | 76.8\% | 8.6 | 25.2 | 71.9\% | 7.9 | 22.7 |
| PRC | -117.4\% |  |  | -115.8\% |  |  |
| Total Delay (pcu/hr) | 256.60 |  |  | 392.66 |  |  |

3.12 As shown in Table 3.3 above, the proposed junction design actually does not provide any improvements to junction capacity, and in fact it worsens capacity significantly, in comparison to the existing junction design. Thus, it can be concluded that the proposed junction design is not feasible.

## Outcome / Conclusions

3.13 The Base Year scenario demonstrated that the existing junction currently operates with sufficient spare capacity in both peak periods, however, the junction is expected to go over capacity in the Future Year scenario. A proposed junction design was tested whereby the public realm and pedestrian crossings at the junction were enhanced, this results in a reduction of two pedestrian crossings. It was found that this design actually causes the junction to perform a lot worse than the existing junction. It is considered that the cause of this is due to there being fewer pedestrian crossings, which means
there are fewer opportunities for traffic phases to run, therefore reducing its efficiency. With the existing junction, there were a number of pedestrian crossings which could run independently of each other, and thus allowing the traffic phases to operate more independently as a result. Subsequently, it can be concluded that there is little that can be done physically to the junction to improve its efficiency and capacity.

## 4 Summary and Conclusions

## Summary

4.1 WYG have been appointed by the Isle of Wight (IOW) Council to undertake a feasibility study for a series of highway junctions located throughout the island, with a particular focus on the towns of Newport, Ryde, Shanklin and Sandown. The study has been carried out to identify traffic management issues impacting on all road users and develop proposals that will form part of a package of schemes to be progressed as part of the Council's Local Implementation Plan (LIP) process.
4.2 IOW Council has identified 15 highway junctions which currently experience traffic issues such as congestion and queuing. The study considers where the main issues lie in relation to traffic movement, road safety, bus operation, pedestrian and cycle provision, public realm, parking provision and servicing. Each of the 15 highway junctions are to be supplemented by a feasibility study report, with traffic modelling software used to test the various proposals in order to identify a range of measures aimed at improving the behaviour and movement of traffic at each junction.
4.3 The Queen's Road / Pellhurst Road / Binstead Road 'T' junction is located on the south-western edge of Ryde, approximately 1 km to Ryde town centre. The A3054 Binstead Road joins Pellhurst Road with Queen's Road forming the northern arm of the ' $T$ ' junction.
4.4 A3054 Binstead Road forms the main route to Fishbourne, whilst Pellhurst Road provides the main access to the southern point of Ryde, and further onto numerous villages and small towns via Upton Road. To the north is Ryde Golf Club and Merrydale, to the east is Ryde School and Upper Chine and the town centre, to the south is Ryde Cemetery, Ryde Academy, Ryde Lawn Tennis and Croquet Club and to the west is Greenmount Community Primary School and Binstead. At present, the junction is known to experience some queuing, of which has been informed by a site visit and traffic video surveys at the junction. It was observed that congestion and queuing is particularly evident on Pellhurst Road.
4.5 The traffic signalling software LinSig V3 was used to model this signalised junction. The modelling was undertaken for two weekday periods considered to be the network peaks in terms of traffic volumes, with the AM peak between 08:00 and 09:00 and the PM peak between 17:00 and 18:00. The Base Year results indicated that the existing junction currently performs well with sufficient spare capacity, however it is noted that the PM peak exhibits lower reserve capacity in comparison to the AM peak. For instance, the junction records a PRC of $46.3 \%$ in the AM peak and a PRC of $12.3 \%$ in the PM peak. In terms of queuing, the highest modelled queues were recorded on Binstead Road, which comprised 10 vehicles.
4.6 A proposed scheme was developed and tested as part of the traffic modelling. This scheme included relocating the pedestrian crossings on Queen's Road and Pellhurst Road, with one of the central pedestrian islands removed, resulting in a reduction of two pedestrian crossings. As well as this, there would be a realignment of the kerb line between Queen's Road and Binstead Road / Pellhurst Road, on the northern and southern side of the carriageway. Mayfield Road would be made one-way eastbound only, with traffic only permitted onto the junction. This scheme is primarily focused on improving the crossing facilities and public realm for pedestrians.
4.7 With the proposed junction design tested, the results demonstrated that the proposed junction design actually does not provide any improvements to junction capacity, and in fact it worsens capacity significantly, in comparison to the existing junction. It is noted that due to there being fewer pedestrian crossings, there are fewer opportunities for traffic phases to run, therefore reducing the junction's efficiency. With the existing junction, there were a number of pedestrian crossings which could run independently of each other, and thus allowing the traffic phases to operate more independently as a result. Therefore, it can be concluded that the proposed junction design is not feasible.

## Conclusions

4.8 As part of this feasibility study, it has been demonstrated from the traffic modelling that there is little that can be done physically to the junction to improve its efficiency and capacity. As a result, it is recommended that the junction should remain in its existing form, as this is deemed the most efficient design for overall junction capacity. It is also recommended that non-physical measures such as extending the cycle time at the junction should be explored as an alternative, in potentially providing additional capacity.

## Appendix A <br> 1:200 DRAWINGS



## Appendix B MODELLING OUTPUT RESULTS

Full Input Data And Results
Full Input Data And Results

## User and Project Details

| Project: | A090129-60 |
| :--- | :--- |
| Title: | HCA Tender IoW |
| Location: |  |
| Additional detail: |  |
| File name: | Junction 8 - Binstead Rd Pellhurst Rd AG - Existing Junction.Isg3x |
| Author: |  |
| Company: |  |
| Address: |  |

## Network Layout Diagram



Full Input Data And Results
Phase Diagram


Full Input Data And Results
Phase Input Data

| Phase Name | Phase Type | Assoc. Phase | Street Min | Cont Min |
| :---: | :---: | :---: | :---: | :---: |
| A | Traffic |  | 7 | 7 |
| B | Traffic |  | 7 | 7 |
| C | Traffic |  | 7 | 7 |
| D | Traffic |  | 7 | 7 |
| E | Traffic |  | 7 | 7 |
| F | Pedestrian |  | 6 | 6 |
| G | Pedestrian |  | 6 | 6 |
| H | Pedestrian |  | 6 | 6 |

Phase Intergreens Matrix


## Phases in Stage

| Stage No. | Phases in Stage |
| :---: | :--- |
| 1 | A B D F |
| 2 | C G H |
| 3 | B C G |

## Stage Diagram



## Phase Delays

| Term. Stage | Start Stage | Phase | Type | Value | Cont value |
| :---: | :---: | :---: | :---: | :---: | :---: |
| There are no Phase Delays defined |  |  |  |  |  |

Prohibited Stage Change

|  | To Stage |  |  |  |
| :--- | :--- | :--- | :--- | :--- |
|  |  | 1 | 2 | 3 |
| From | 1 |  | 8 | 8 |
| Stage | 2 | 7 |  | 5 |
|  |  | 3 | 7 | 7 |

## Full Input Data And Results

## Give-Way Lane Input Data

| Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Movement | Max Flow <br> when <br> Giving Way <br> (PCU/Hr) | Min Flow <br> when <br> Giving Way <br> (PCU/Hr) | Opposing <br> Lane | Opp. Lane <br> Coeff. | Opp. <br> Mvmnts. | Right Turn <br> Storage (PCU) | Non-Blocking <br> Storage <br> (PCU) | RTF | Right Turn <br> Move up (s) |
| Mn Intergreen <br> (PCU) |  |  |  |  |  |  |  |  |  |  |
| $1 / 2$ <br> (Binstead Rd Entry) | $8 / 1$ (Right) | 1439 | 0 | $3 / 1$ | 1.09 | All | - | - | - | - |

Full Input Data And Results
Lane Input Data
Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd

| Lane | Lane <br> Type | Phases | Start Disp. | End Disp. | Physical Length (PCU) | Sat Flow Type | Def User Saturation Flow (PCU/Hr) | Lane Width (m) | Gradient | Nearside Lane | Turns | Turning Radius (m) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $1 / 1$ <br> (Binstead Rd Entry) | U | B | 2 | 3 | 2.0 | Geom | - | 2.37 | 0.00 | Y | Arm 5 Left | 15.00 |
| $1 / 2$ <br> (Binstead Rd Entry) | 0 | A | 2 | 3 | 60.0 | Geom | - | 2.50 | 0.00 | Y | Arm 6 <br> Ahead | Inf |
|  |  |  |  |  |  |  |  |  |  |  | Arm 8 Right | 5.00 |
| $2 / 1$ <br> (Queens Rd Entry) | U | C | 2 | 3 | 60.0 | Geom | - | 2.60 | 0.00 | Y | Arm 4 Right | 21.00 |
|  |  |  |  |  |  |  |  |  |  |  | Arm 6 <br> Left | 12.50 |
|  |  |  |  |  |  |  |  |  |  |  | Arm 8 Ahead | Inf |
| $3 / 1$ <br> (Pellhurst Rd Entry) | U | D | 2 | 3 | 60.0 | Geom | - | 2.90 | 0.00 | Y | Arm 4 Ahead | Inf |
|  |  |  |  |  |  |  |  |  |  |  | Arm 8 Left | 4.00 |
| 4/1 <br> (Binstead Rd Exit) | U |  | 2 | 3 | 60.0 | Inf | - | - | - | - | - | - |
| $5 / 1$ (Queens Rd Exit) | U |  | 2 | 3 | 60.0 | Inf | - | - | - | - | - | - |
| $\begin{gathered} 6 / 1 \\ \text { (Pellhurst Rd } \\ \text { Exit) } \end{gathered}$ | U |  | 2 | 3 | 60.0 | Inf | - | - | - | - | - | - |
| 7/1 (Mayfield Rd Entry) | U |  | 2 | 3 | 60.0 | Inf | - | - | - | - | - | - |
| $\begin{gathered} 8 / 1 \\ \text { (Mayfield Rd } \\ \text { Exit) } \end{gathered}$ | U |  | 2 | 3 | $60.0$ | Inf | - | - | - | - | - | - |

Traffic Flow Groups

| Flow Group | Start Time | End Time | Duration | Formula |
| :---: | :---: | :---: | :---: | :---: |
| 1: '2016 - AM' | $08: 00$ | $09: 00$ | $01: 00$ |  |
| 2: '2016 - PM' | $17: 00$ | $18: 00$ | $01: 00$ |  |
| 3: '2034- AM - Base' | $08: 00$ | $09: 00$ | $01: 00$ |  |
| 4: '2034 - PM - Base' | $17: 00$ | $18: 00$ | $01: 00$ |  |
| 5: '20XX - AM - DS' | $08: 00$ | $09: 00$ | $01: 00$ |  |
| 6: '20XX - PM - DS' | $17: 00$ | $18: 00$ | $01: 00$ |  |
| 7: '2017 - AM Base' | $08: 00$ | $09: 00$ | $01: 00$ |  |
| 8: '2017 - PM Base' | $17: 00$ | $18: 00$ | $01: 00$ |  |

Scenario 1: '2017 AM Base' (FG7: '2017 - AM Base', Plan 1: 'Network Control Plan 1') Traffic Flows, Desired
Desired Flow :

|  | Destination |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Origin |  | A | B | C | D | Tot. |
|  | A | 0 | 298 | 375 | 0 | 673 |
|  | B | 243 | 0 | 11 | 0 | 254 |
|  | C | 430 | 0 | 0 | 0 | 430 |
|  | D | 0 | 0 | 0 | 0 | 0 |
|  | Tot. | 673 | 298 | 386 | 0 | 1357 |

## Traffic Lane Flows

| Lane | Scenario 1: <br> 2017 AM Base |
| :---: | :---: |
| Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd |  |
| $1 / 1$ <br> (short) | 298 |
| $1 / 2$ <br> (with short) | $673(\mathrm{In})$ <br> $375($ Out $)$ |
| $2 / 1$ | 254 |
| $3 / 1$ | 430 |
| $4 / 1$ | 673 |
| $5 / 1$ | 298 |
| $6 / 1$ | 386 |
| $7 / 1$ | 0 |
| $8 / 1$ | 0 |

Full Input Data And Results
Lane Saturation Flows

| Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Lane Width (m) | Gradient | Nearside Lane | Allowed Turns | Turning Radius (m) | Turning Prop. | Sat Flow (PCU/Hr) | Flared Sat Flow (PCU/Hr) |
| $1 / 1$ (Binstead Rd Entry) <br> 1/2 <br> (Binstead Rd Entry) | 2.37 | 0.00 | Y |  | 15.00 | 100.0 \% | 1684 | 1684 |
|  | 2.50 | 0.00 | Y | Arm 6 Ahead | Inf | 100.0 \% | 1865 | 1865 |
|  |  |  |  | Arm 8 Right | 5.00 | 0.0 \% |  |  |
| $2 / 1$(Queens Rd Entry) | 2.60 | 0.00 | Y | Arm 4 Right | 21.00 | 95.7 \% | 1747 | 1747 |
|  |  |  |  | Arm 6 Left | 12.50 | 4.3 \% |  |  |
|  |  |  |  | Arm 8 Ahead | Inf | 0.0 \% |  |  |
| $\begin{gathered} 3 / 1 \\ \text { (Pellhurst Rd Entry) } \end{gathered}$ | 2.90 | 0.00 | Y | Arm 4 Ahead | Inf | 100.0 \% | 1905 | 1905 |
|  |  |  |  | Arm 8 Left | 4.00 | 0.0 \% |  |  |
| $\begin{gathered} 4 / 1 \\ \text { (Binstead Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 5 / 1 \\ \text { (Queens Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 6 / 1 \\ \text { (Pellhurst Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} \text { (Mayfield Rd Entry Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 8 / 1 \\ \text { (Mayfield Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |

Scenario 2: '2017 PM Base ' (FG8: '2017 - PM Base', Plan 1: 'Network Control Plan 1') Traffic Flows, Desired
Desired Flow :

|  | Destination |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Origin |  | A | B | C | D | Tot. |  |
|  | A | 0 | 398 | 425 | 0 | 823 |  |
|  | B | 378 | 0 | 21 | 0 | 399 |  |
|  | C | 424 | 0 | 0 | 0 | 424 |  |
|  | D | 0 | 0 | 0 | 0 | 0 |  |
|  | Tot. | 802 | 398 | 446 | 0 | 1646 |  |

Full Input Data And Results
Traffic Lane Flows

| Lane | Scenario 2: <br> 2017 PM Base |
| :---: | :---: |
| Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd |  |
| $1 / 1$ <br> (short) | 398 |
| $1 / 2$ <br> (with short) | $823(\mathrm{ln})$ <br> $425($ Out $)$ |
| $2 / 1$ | 399 |
| $3 / 1$ | 424 |
| $4 / 1$ | 802 |
| $5 / 1$ | 398 |
| $6 / 1$ | 446 |
| $7 / 1$ | 0 |
| $8 / 1$ | 0 |

## Lane Saturation Flows

| Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Lane Width (m) | Gradient | Nearside Lane | Allowed Turns | Turning Radius (m) | Turning Prop. | Sat Flow (PCU/Hr) | Flared Sat Flow (PCU/Hr) |
| $\begin{gathered} 1 / 1 \\ \text { (Binstead Rd Entry) } \end{gathered}$ | 2.37 | 0.00 | Y | Arm 5 Left | 15.00 | 100.0 \% | 1684 | 1684 |
| $1 / 2$(Binstead Rd Entry) | 2.50 | 0.00 | Y | Arm 6 Ahead | Inf | 100.0 \% | 1865 | 1865 |
|  |  |  |  | Arm 8 Right | 5.00 | 0.0 \% |  |  |
| 2/1(Queens Rd Entry) | 2.60 | 0.00 | Y | Arm 4 Right | 21.00 | 94.7 \% | 1746 | 1746 |
|  |  |  |  | Arm 6 Left | 12.50 | 5.3 \% |  |  |
|  |  |  |  | Arm 8 Ahead | Inf | 0.0 \% |  |  |
| $\begin{gathered} 3 / 1 \\ \text { (Pellhurst Rd Entry) } \end{gathered}$ | 2.90 | 0.00 | Y | Arm 4 Ahead | Inf | 100.0 \% | 1905 | 1905 |
|  |  |  |  | Arm 8 Left | 4.00 | 0.0 \% |  |  |
| $\begin{gathered} 4 / 1 \\ \text { (Binstead Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 5 / 1 \\ \text { (Queens Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 6 / 1 \\ \text { (Pellhurst Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 7 / 1 \\ \text { (Mayfield Rd Entry Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 8 / 1 \\ \text { (Mayfield Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |

Full Input Data And Results
Scenario 3: '2034 AM Base' (FG3: '2034- AM - Base', Plan 1: 'Network Control Plan 1') Traffic Flows, Desired
Desired Flow :

|  | Destination |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Origin |  | A | B | C | D | Tot. |
|  | A | 0 | 363 | 458 | 11 | 832 |
|  | B | 267 | 0 | 13 | 29 | 309 |
|  | C | 404 | 0 | 0 | 132 | 536 |
|  | D | 0 | 0 | 0 | 0 | 0 |
|  | Tot. | 671 | 363 | 471 | 172 | 1677 |

## Traffic Lane Flows

| Lane | Scenario 3: <br> 2034 AM Base |
| :---: | :---: |
| Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd |  |
| $1 / 1$ <br> (short) | 363 |
| $1 / 2$ |  |
| (with short) |  |
| $2 / 1$ | $832(\mathrm{ln})$ <br> $469($ Out $)$ <br> $3 / 1$ |
| $4 / 1$ | 539 |
| $5 / 1$ | 671 |
| $6 / 1$ | 363 |
| $7 / 1$ | 471 |
| $8 / 1$ | 0 |

Full Input Data And Results
Lane Saturation Flows

| Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Lane Width (m) | Gradient | Nearside Lane | Allowed Turns | Turning Radius (m) | Turning Prop. | Sat Flow (PCU/Hr) | Flared Sat Flow (PCU/Hr) |
| $1 / 1$ (Binstead Rd Entry) <br> 1/2 <br> (Binstead Rd Entry) | 2.37 | 0.00 | Y |  | 15.00 | 100.0 \% | 1684 | 1684 |
|  | 2.50 | 0.00 | Y | Arm 6 Ahead | Inf | 97.7\% | 1852 | 1852 |
|  |  |  |  | Arm 8 Right | 5.00 | 2.3 \% |  |  |
| $2 / 1$(Queens Rd Entry) | 2.60 | 0.00 | Y | Arm 4 Right | 21.00 | 86.4 \% | 1758 | 1758 |
|  |  |  |  | Arm 6 Left | 12.50 | 4.2 \% |  |  |
|  |  |  |  | Arm 8 Ahead | Inf | 9.4 \% |  |  |
| $\begin{gathered} 3 / 1 \\ \text { (Pellhurst Rd Entry) } \end{gathered}$ | 2.90 | 0.00 | Y | Arm 4 Ahead | Inf | 75.4 \% | 1744 | 1744 |
|  |  |  |  | Arm 8 Left | 4.00 | 24.6 \% |  |  |
| $\begin{gathered} 4 / 1 \\ \text { (Binstead Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 5 / 1 \\ \text { (Queens Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 6 / 1 \\ \text { (Pellhurst Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 7 / 1 \\ \text { (Mayfield Rd Entry Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 8 / 1 \\ \text { (Mayfield Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |

Scenario 4: '2034 PM Base' (FG4: '2034 - PM - Base', Plan 1: 'Network Control Plan 1') Traffic Flows, Desired Desired Flow :

|  | Destination |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Origin |  | A | B | C | D | Tot. |
|  | A | 0 | 485 | 518 | 20 | 1023 |
|  | B | 418 | 0 | 25 | 42 | 485 |
|  | C | 430 | 0 | 0 | 86 | 516 |
|  | D | 1 | 0 | 0 | 0 | 1 |
|  | Tot. | 849 | 485 | 543 | 148 | 2025 |

Full Input Data And Results
Traffic Lane Flows

| Lane | Scenario 4: <br> 2034 PM Base |
| :---: | :---: |
| Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd |  |
| $1 / 1$ <br> (short) | 485 |
| $1 / 2$ <br> (with short) | $1023(\mathrm{ln})$ <br> $538($ Out ) |
| $2 / 1$ | 485 |
| $3 / 1$ | 516 |
| $4 / 1$ | 849 |
| $5 / 1$ | 485 |
| $6 / 1$ | 543 |
| $7 / 1$ | 1 |
| $8 / 1$ | 148 |

## Lane Saturation Flows

| Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Lane Width (m) | Gradient | Nearside Lane | Allowed Turns | Turning Radius (m) | Turning Prop. | Sat Flow (PCU/Hr) | Flared Sat Flow (PCU/Hr) |
| $\begin{gathered} 1 / 1 \\ \text { (Binstead Rd Entry) } \end{gathered}$ | 2.37 | 0.00 | Y | Arm 5 Left | 15.00 | 100.0 \% | 1684 | 1684 |
| $1 / 2$(Binstead Rd Entry) | 2.50 | 0.00 | Y | Arm 6 Ahead | Inf | 96.3 \% | 1844 | 1844 |
|  |  |  |  | Arm 8 Right | 5.00 | 3.7 \% |  |  |
| 2/1(Queens Rd Entry) | 2.60 | 0.00 | Y | Arm 4 Right | 21.00 | 86.2 \% | 1756 | 1756 |
|  |  |  |  | Arm 6 Left | 12.50 | 5.2 \% |  |  |
|  |  |  |  | Arm 8 Ahead | Inf | 8.7 \% |  |  |
| $\begin{gathered} 3 / 1 \\ \text { (Pellhurst Rd Entry) } \end{gathered}$ | 2.90 | 0.00 | Y | Arm 4 Ahead | Inf | 83.3 \% | 1793 | 1793 |
|  |  |  |  | Arm 8 Left | 4.00 | 16.7 \% |  |  |
| $\begin{gathered} 4 / 1 \\ \text { (Binstead Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 5 / 1 \\ \text { (Queens Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 6 / 1 \\ \text { (Pellhurst Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 7 / 1 \\ \text { (Mayfield Rd Entry Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 8 / 1 \\ \text { (Mayfield Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |

Full Input Data And Results
Scenario 5: '2034 AM Base (80 seconds)' (FG3: '2034- AM - Base', Plan 1: 'Network Control Plan 1') Traffic Flows, Desired Desired Flow :

|  | Destination |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Origin |  | A | B | C | D | Tot. |
|  | A | 0 | 363 | 458 | 11 | 832 |
|  | B | 267 | 0 | 13 | 29 | 309 |
|  | C | 404 | 0 | 0 | 132 | 536 |
|  | D | 0 | 0 | 0 | 0 | 0 |
|  | Tot. | 671 | 363 | 471 | 172 | 1677 |

## Traffic Lane Flows

| Lane | Scenario 5: <br> 2034 AM Base (80 seconds) |
| :---: | :---: |
| Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd |  |
| $1 / 1$ <br> (short) | 363 |
| $1 / 2$ <br> (with short) <br> $2 / 1$ | $832(\mathrm{ln})$ <br> $469($ Out $)$ <br> 309 |
| $3 / 1$ | 536 |
| $4 / 1$ | 671 |
| $5 / 1$ | 363 |
| $6 / 1$ | 471 |
| $7 / 1$ | 0 |
| $8 / 1$ | 172 |

Full Input Data And Results
Lane Saturation Flows

| Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Lane Width (m) | Gradient | Nearside Lane | Allowed Turns | Turning Radius (m) | Turning Prop. | Sat Flow (PCU/Hr) | Flared Sat Flow (PCU/Hr) |
| $1 / 1$(Binstead Rd Entry)$1 / 2$(Binstead Rd Entry) | 2.37 | 0.00 | Y |  | 15.00 | 100.0 \% | 1684 | 1684 |
|  | 2.50 | 0.00 | Y | Arm 6 Ahead | Inf | 97.7\% | 1852 | 1852 |
|  |  |  |  | Arm 8 Right | 5.00 | 2.3 \% |  |  |
| $2 / 1$(Queens Rd Entry) | 2.60 | 0.00 | Y | Arm 4 Right | 21.00 | 86.4 \% | 1758 | 1758 |
|  |  |  |  | Arm 6 Left | 12.50 | 4.2 \% |  |  |
|  |  |  |  | Arm 8 Ahead | Inf | 9.4 \% |  |  |
| $\begin{gathered} 3 / 1 \\ \text { (Pellhurst Rd Entry) } \end{gathered}$ | 2.90 | 0.00 | Y | Arm 4 Ahead | Inf | 75.4 \% | 1744 | 1744 |
|  |  |  |  | Arm 8 Left | 4.00 | 24.6 \% |  |  |
| $\begin{gathered} 4 / 1 \\ (\text { Binstead Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 5 / 1 \\ \text { (Queens Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 6 / 1 \\ \text { (Pellhurst Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 7 / 1 \\ \text { (Mayfield Rd Entry Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 8 / 1 \\ \text { (Mayfield Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |

Scenario 6: '2034 PM Base (80 seconds)' (FG4: '2034 - PM - Base', Plan 1: 'Network Control Plan 1') Traffic Flows, Desired Desired Flow :

|  | Destination |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Origin |  | A | B | C | D | Tot. |  |
|  | A | 0 | 485 | 518 | 20 | 1023 |  |
|  | B | 418 | 0 | 25 | 42 | 485 |  |
|  | C | 430 | 0 | 0 | 86 | 516 |  |
|  | D | 1 | 0 | 0 | 0 | 1 |  |

Full Input Data And Results
Traffic Lane Flows

| Lane | Scenario 6: <br> 2034 PM Base (80 seconds) |
| :---: | :---: |
| Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd |  |
| $1 / 1$ <br> (short) | 485 |
| $1 / 2$ <br> (with short) | $1023(\mathrm{In})$ <br> $538($ Out) |
| $2 / 1$ | 485 |
| $3 / 1$ | 516 |
| $4 / 1$ | 849 |
| $5 / 1$ | 485 |
| $6 / 1$ | 543 |
| $7 / 1$ | 1 |
| $8 / 1$ | 148 |

## Lane Saturation Flows

| Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Lane Width (m) | Gradient | Nearside Lane | Allowed Turns | Turning Radius (m) | Turning Prop. | Sat Flow (PCU/Hr) | Flared Sat Flow (PCU/Hr) |
| $\begin{gathered} 1 / 1 \\ \text { (Binstead Rd Entry) } \end{gathered}$ | 2.37 | 0.00 | Y | Arm 5 Left | 15.00 | 100.0 \% | 1684 | 1684 |
| $1 / 2$(Binstead Rd Entry) | 2.50 | 0.00 | Y | Arm 6 Ahead | Inf | 96.3 \% | 1844 | 1844 |
|  |  |  |  | Arm 8 Right | 5.00 | 3.7 \% |  |  |
| 2/1(Queens Rd Entry) | 2.60 | 0.00 | Y | Arm 4 Right | 21.00 | 86.2 \% | 1756 | 1756 |
|  |  |  |  | Arm 6 Left | 12.50 | 5.2 \% |  |  |
|  |  |  |  | Arm 8 Ahead | Inf | 8.7 \% |  |  |
| $\begin{gathered} 3 / 1 \\ \text { (Pellhurst Rd Entry) } \end{gathered}$ | 2.90 | 0.00 | Y | Arm 4 Ahead | Inf | 83.3 \% | 1793 | 1793 |
|  |  |  |  | Arm 8 Left | 4.00 | 16.7 \% |  |  |
| $\begin{gathered} 4 / 1 \\ \text { (Binstead Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 5 / 1 \\ \text { (Queens Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 6 / 1 \\ \text { (Pellhurst Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 7 / 1 \\ \text { (Mayfield Rd Entry Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 8 / 1 \\ \text { (Mayfield Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |

Full Input Data And Results
Scenario 1: '2017 AM Base' (FG7: '2017 - AM Base', Plan 1: 'Network Control Plan 1')
Stage Sequence Diagram


Stage Timings

| Stage | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
| :---: | :---: | :---: | :---: |
| Duration | 29 | 5 | 1 |
| Change Point | 0 | 36 | 49 |

Signal Timings Diagram


Time in cycle (sec)


## Full Input Data And Results

## Network Results

| Item | Lane Description | Lane Type | Controller Stream | Position In Filtered Route | Full Phase | Arrow Phase | Num Greens | Total Green <br> (s) | Arrow Green (s) | Demand Flow (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | Deg Sat <br> (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: HCA Tender loW | - | - | N/A | - | - |  | - | - | - | - | - | - | 61.5\% |
| Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd | - | - | N/A | - | - |  | - | - | - | - | - | - | 61.5\% |
| 1/2+1/1 | Binstead Rd Entry Left Ahead Right | O+U | N/A | N/A | AB |  | 1 | 30:37 | - | 673 | 1865:1684 | 1109 | 60.7\% |
| 2/1 | Queens Rd Entry Right Left Ahead | U | N/A | N/A | C |  | 1 | 12 | - | 254 | 1747 | 413 | 61.5\% |
| 3/1 | Pellhurst Rd Entry Ahead Left | U | N/A | N/A | D |  | 1 | 29 | - | 430 | 1905 | 1039 | 41.4\% |
| 4/1 | Binstead Rd Exit | U | N/A | N/A | - |  | - | - | - | 673 | Inf | Inf | 0.0\% |
| 5/1 | Queens Rd Exit | U | N/A | N/A | - |  | - | - | - | 298 | Inf | Inf | 0.0\% |
| 6/1 | $\underset{\text { Exit }}{\text { Pellhurst Rd }}$ | U | N/A | N/A | - |  | - | - | - | 386 | Inf | Inf | 0.0\% |
| 7/1 | Mayfield Rd Entry Left Ahead Right | U | N/A | N/A | - |  | - | - | - | 0 | Inf | Inf | 0.0\% |
| 8/1 | Mayfield Rd Exit | U | N/A | N/A | - |  | - | - | - | 0 | Inf | Inf | 0.0\% |
| Ped Link: P1 | Unnamed Ped Link | - | N/A | - | F |  | 1 | 31 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P2 | Unnamed Ped Link | - | N/A | - | G |  | 1 | 11 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P3 | Unnamed Ped Link | - | N/A | - | F |  | 1 | 31 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P4 | Unnamed Ped Link | - | N/A | - | H |  | 1 | 6 | - | 0 | - | 0 | 0.0\% |

Full Input Data And Results

| Item | Arriving (pcu) | Leaving (pcu) | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Uniform Delay (pcuHr) | Rand + Oversat Delay (pcuHr) | Storage Area Uniform Delay (pcuHr) | Total Delay (pcuHr) | Av. Delay Per PCU (s/pcu) | Max. Back of Uniform Queue (pcu) | Rand + Oversat Queue (pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: HCA Tender IoW | - | - | 0 | 0 | 0 | 3.3 | 1.9 | 0.0 | 5.2 | - | - | - | - |
| Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd | - | - | 0 | 0 | 0 | 3.3 | 1.9 | 0.0 | 5.2 | - | - | - | - |
| 1/2+1/1 | 673 | 673 | 0 | 0 | 0 | 1.1 | 0.8 | - | 1.8 | 9.8 | 4.6 | 0.8 | 5.4 |
| 2/1 | 254 | 254 | - | - | - | 1.3 | 0.8 | - | 2.1 | 30.0 | 3.5 | 0.8 | 4.2 |
| 3/1 | 430 | 430 | - | - | - | 0.9 | 0.4 | - | 1.2 | 10.3 | 3.8 | 0.4 | 4.2 |
| 4/1 | 673 | 673 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 5/1 | 298 | 298 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 6/1 | 386 | 386 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 7/1 | 0 | 0 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 8/1 | 0 | 0 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Ped Link: P1 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P2 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P3 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P4 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| C1 |  |  | PRC for Signalled Lanes (\%): PRC Over All Lanes (\%): |  | $\begin{aligned} & 46.3 \\ & 46.3 \end{aligned}$ | Total Delay for Signalled Lanes (pcuHr): <br> Total Delay Over All Lanes(pcuHr): |  |  | Cycle Time (s): 55 |  |  |  |  |

Full Input Data And Results
Scenario 2: '2017 PM Base ' (FG8: '2017 - PM Base', Plan 1: 'Network Control Plan 1')
Stage Sequence Diagram


## Stage Timings

| Stage | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
| :---: | :---: | :---: | :---: |
| Duration | 26 | 5 | 4 |
| Change Point | 0 | 33 | 46 |

Signal Timings Diagram


Full Input Data And Results

## Network Layout Diagram



Network Results

| Item | Lane Description | Lane Type | Controller Stream | Position In Filtered Route | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow Green (s) | Demand Flow (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | Deg Sat <br> (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: HCA <br> Tender IoW | - | - | N/A | - | - |  | - | - | - | - | - | - | 80.1\% |
| Queens <br> Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd | - | - | N/A | - | - |  | - | - | - | - | - | - | 80.1\% |
| 1/2+1/1 | Binstead Rd Entry Left Ahead Right | O+U | N/A | N/A | A B |  | 1 | 27:37 | - | 823 | 1865:1684 | 1027 | 80.1\% |
| 2/1 | Queens Rd Entry Right Left Ahead | U | N/A | N/A | C |  | 1 | 15 | - | 399 | 1746 | 508 | 78.6\% |
| 3/1 | Pellhurst Rd Entry Ahead Left | U | N/A | N/A | D |  | 1 | 26 | - | 424 | 1905 | 935 | 45.3\% |
| 4/1 | Binstead Rd Exit | U | N/A | N/A | - |  | - | - | - | 802 | Inf | Inf | 0.0\% |
| 5/1 | Queens Rd Exit | U | N/A | N/A | - |  | - | - | - | 398 | Inf | Inf | 0.0\% |
| 6/1 | Pellhurst Rd Exit | U | N/A | N/A | - |  | - | - | - | 446 | Inf | Inf | 0.0\% |
| 7/1 | Mayfield Rd Entry Left Ahead Right | U | N/A | N/A | - |  | - | - | - | 0 | Inf | Inf | 0.0\% |
| 8/1 | Mayfield Rd Exit | U | N/A | N/A | - |  | - | - | - | 0 | Inf | Inf | 0.0\% |
| Ped Link: P1 | Unnamed Ped Link | - | N/A | - | F |  | 1 | 28 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P2 | Unnamed Ped Link | - | N/A | - | G |  | 1 | 14 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P3 | Unnamed Ped Link | - | N/A | - | F |  | 1 | 28 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P4 | Unnamed Ped Link | - | N/A | - | H |  | 1 | 6 | - | 0 | - | 0 | 0.0\% |

Full Input Data And Results

| Item | Arriving (pcu) | Leaving (pcu) | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Uniform Delay (pcuHr) | Rand + Oversat Delay (pcuHr) | Storage Area Uniform Delay (pcuHr) | Total Delay (pcuHr) | Av. Delay Per PCU (s/pcu) | Max. Back of Uniform Queue (pcu) | Rand + Oversat Queue (pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: HCA Tender IoW | - | - | 0 | 0 | 0 | 4.9 | 4.2 | 0.0 | 9.1 | - | - | - | - |
| Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd | - | - | 0 | 0 | 0 | 4.9 | 4.2 | 0.0 | 9.1 | - | - | - | - |
| 1/2+1/1 | 823 | 823 | 0 | 0 | 0 | 1.9 | 2.0 | - | 3.8 | 16.8 | 8.0 | 2.0 | 9.9 |
| 2/1 | 399 | 399 | - | - | - | 2.0 | 1.8 | - | 3.8 | 33.9 | 5.5 | 1.8 | 7.3 |
| 3/1 | 424 | 424 | - | - | - | 1.1 | 0.4 | - | 1.5 | 12.7 | 4.2 | 0.4 | 4.7 |
| 4/1 | 802 | 802 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 5/1 | 398 | 398 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 6/1 | 446 | 446 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 7/1 | 0 | 0 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 8/1 | 0 | 0 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Ped Link: P1 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P2 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P3 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P4 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| C1 |  |  | PRC for Signalled Lanes (\%): PRC Over All Lanes (\%): |  | $\begin{aligned} & 12.3 \\ & 12.3 \end{aligned}$ | Total Delay for Signalled Lanes (pcuHr): Total Delay Over All Lanes(pcuHr): |  |  | Cycle Time (s): 55 |  |  |  |  |

Full Input Data And Results
Scenario 3: '2034 AM Base' (FG3: '2034- AM - Base', Plan 1: 'Network Control Plan 1')
Stage Sequence Diagram


## Stage Timings

| Stage | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
| :---: | :---: | :---: | :---: |
| Duration | 30 | 5 | 0 |
| Change Point | 0 | 37 | 50 |

Signal Timings Diagram



Network Results

| Item | Lane Description | Lane Type | Controller Stream | Position In Filtered Route | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow Green (s) | Demand Flow (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | Deg Sat <br> (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: HCA <br> Tender IoW | - | - | N/A | - | - |  | - | - | - | - | - | - | 92.9\% |
| Queens <br> Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd | - | - | N/A | - | - |  | - | - | - | - | - | - | 92.9\% |
| 1/2+1/1 | Binstead Rd Entry Left Ahead Right | O+U | N/A | N/A | A B |  | 1 | 31:37 | - | 832 | 1852:1684 | 895 | 92.9\% |
| 2/1 | Queens Rd Entry Right Left Ahead | U | N/A | N/A | C |  | 1 | 11 | - | 309 | 1758 | 384 | 80.6\% |
| 3/1 | Pellhurst Rd Entry Ahead Left | U | N/A | N/A | D |  | 1 | 30 | - | 536 | 1744 | 983 | 54.5\% |
| 4/1 | Binstead Rd Exit | U | N/A | N/A | - |  | - | - | - | 671 | Inf | Inf | 0.0\% |
| 5/1 | Queens Rd Exit | U | N/A | N/A | - |  | - | - | - | 363 | Inf | Inf | 0.0\% |
| 6/1 | Pellhurst Rd Exit | U | N/A | N/A | - |  | - | - | - | 471 | Inf | Inf | 0.0\% |
| 7/1 | Mayfield Rd Entry Left Ahead Right | U | N/A | N/A | - |  | - | - | - | 0 | Inf | Inf | 0.0\% |
| 8/1 | Mayfield Rd Exit | U | N/A | N/A | - |  | - | - | - | 172 | Inf | Inf | 0.0\% |
| Ped Link: P1 | Unnamed Ped Link | - | N/A | - | F |  | 1 | 32 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P2 | Unnamed Ped Link | - | N/A | - | G |  | 1 | 10 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P3 | Unnamed Ped Link | - | N/A | - | F |  | 1 | 32 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P4 | Unnamed Ped Link | - | N/A | - | H |  | 1 | 6 | - | 0 | - | 0 | 0.0\% |

Full Input Data And Results

| Item | Arriving (pcu) | Leaving (pcu) | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Uniform Delay (pcuHr) | Rand + Oversat Delay (pcuHr) | Storage Area Uniform Delay (pcuHr) | Total Delay (pcuHr) | Av. Delay Per PCU (s/pcu) | Max. Back of Uniform Queue (pcu) | Rand + Oversat Queue (pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: HCA Tender loW | - | - | 9 | 2 | 0 | 5.3 | 8.1 | 0.0 | 13.4 | - | - | - | - |
| Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd | - | - | 9 | 2 | 0 | 5.3 | 8.1 | 0.0 | 13.4 | - | - | - | - |
| 1/2+1/1 | 832 | 832 | 9 | 2 | 0 | 2.4 | 5.6 | - | 8.0 | 34.4 | 10.2 | 5.6 | 15.8 |
| 2/1 | 309 | 309 | - | - | - | 1.8 | 2.0 | - | 3.7 | 43.3 | 4.5 | 2.0 | 6.4 |
| 3/1 | 536 | 536 | - | - | - | 1.1 | 0.6 | - | 1.7 | 11.6 | 5.1 | 0.6 | 5.7 |
| 4/1 | 671 | 671 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 5/1 | 363 | 363 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 6/1 | 471 | 471 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 7/1 | 0 | 0 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 8/1 | 172 | 172 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Ped Link: P1 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P2 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P3 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P4 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| C1 |  |  | PRC for Signalled Lanes (\%): PRC Over All Lanes (\%): |  | $\begin{aligned} & -3.2 \\ & -3.2 \end{aligned}$ | Total Delay for Signalled Lanes (pcuHr): Total Delay Over All Lanes(pcuHr): |  |  | Cycle Time (s): 55 |  |  |  |  |

Full Input Data And Results
Scenario 4: '2034 PM Base' (FG4: '2034 - PM - Base', Plan 1: 'Network Control Plan 1')
Stage Sequence Diagram


## Stage Timings

| Stage | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
| :---: | :---: | :---: | :---: |
| Duration | 29 | 5 | 1 |
| Change Point | 0 | 36 | 49 |

Signal Timings Diagram



Network Results

| Item | Lane Description | Lane Type | Controller Stream | Position In Filtered Route | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow Green (s) | Demand Flow (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | $\begin{aligned} & \text { Deg Sat } \\ & \text { (\%) } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: HCA <br> Tender loW | - | - | N/A | - | - |  | - | - | - | - | - | - | 116.9\% |
| Queens <br> Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd | - | - | N/A | - | - |  | - | - | - | - | - | - | 116.9\% |
| 1/2+1/1 | Binstead Rd Entry Left Ahead Right | O+U | N/A | N/A | A B |  | 1 | 30:37 | - | 1023 | 1844:1684 | 889 | 115.0\% |
| 2/1 | Queens Rd Entry Right Left Ahead | U | N/A | N/A | C |  | 1 | 12 | - | 485 | 1756 | 415 | 116.9\% |
| 3/1 | Pellhurst Rd Entry Ahead Left | U | N/A | N/A | D |  | 1 | 29 | - | 516 | 1793 | 978 | 52.8\% |
| 4/1 | Binstead Rd Exit | U | N/A | N/A | - |  | - | - | - | 849 | Inf | Inf | 0.0\% |
| 5/1 | Queens Rd Exit | U | N/A | N/A | - |  | - | - | - | 485 | Inf | Inf | 0.0\% |
| 6/1 | $\underset{\text { Exit }}{\text { Pellhurst Rd }}$ | U | N/A | N/A | - |  | - | - | - | 543 | Inf | Inf | 0.0\% |
| 7/1 | Mayfield Rd Entry Left Ahead Right | U | N/A | N/A | - |  | - | - | - | 1 | Inf | Inf | 0.0\% |
| 8/1 | Maytield Rd Exit | U | N/A | N/A | - |  | - | - | - | 148 | Inf | Inf | 0.0\% |
| Ped Link: P1 | Unnamed Ped Link | - | N/A | - | F |  | 1 | 31 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P2 | Unnamed Ped Link | - | N/A | - | G |  | 1 | 11 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P3 | Unnamed Ped Link | - | N/A | - | F |  | 1 | 31 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P4 | Unnamed Ped Link | - | N/A | - | H |  | 1 | 6 | - | 0 | - | 0 | 0.0\% |

Full Input Data And Results

| Item | Arriving (pcu) | Leaving (pcu) | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Uniform Delay (pcuHr) | Rand + Oversat Delay (pcuHr) | Storage Area Uniform Delay (pcuHr) | Total Delay (pcuHr) | Av. Delay <br> Per PCU <br> (s/pcu) | Max. Back of Uniform Queue (pcu) | Rand + Oversat Queue (pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: HCA Tender loW | - | - | 14 | 4 | 0 | 12.6 | 109.2 | 0.0 | 121.8 | - | - | - | - |
| Queens <br> Rd/Pellhurst <br> Rd/Mayfield <br> Rd/Binstead Rd | - | - | 14 | 4 | 0 | 12.6 | 109.2 | 0.0 | 121.8 | - | - | - | - |
| 1/2+1/1 | 1023 | 889 | 14 | 4 | 0 | 6.4 | 70.5 | - | 76.9 | 270.5 | 16.4 | 70.5 | 86.9 |
| 2/1 | 485 | 415 | - | - | - | 5.0 | 38.2 | - | 43.2 | 320.6 | 9.1 | 38.2 | 47.3 |
| 3/1 | 516 | 516 | - | - | - | 1.1 | 0.6 | - | 1.7 | 11.9 | 5.0 | 0.6 | 5.6 |
| 4/1 | 789 | 789 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 5/1 | 422 | 422 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 6/1 | 472 | 472 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 7/1 | 1 | 1 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 8/1 | 139 | 139 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Ped Link: P1 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P2 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P3 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P4 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| C1 |  |  | PRC for Signalled Lanes (\%): PRC Over All Lanes (\%): |  | $\begin{aligned} & -29.8 \\ & -29.8 \end{aligned}$ | Total Delay for Signalled Lanes (pcuHr): <br> Total Delay Over All Lanes(pcuHr): |  |  | Cycle Time (s): 55 |  |  |  |  |

Full Input Data And Results
Scenario 5: '2034 AM Base (80 seconds)' (FG3: '2034- AM - Base', Plan 1: 'Network Control Plan 1')
Stage Sequence Diagram


## Stage Timings

| Stage | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
| :---: | :---: | :---: | :---: |
| Duration | 50 | 5 | 5 |
| Change Point | 0 | 57 | 70 |

Signal Timings Diagram



Network Results

| Item | Lane Description | Lane Type | Controller Stream | Position In Filtered Route | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow Green (s) | Demand Flow (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | Deg Sat <br> (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: HCA <br> Tender IoW | - | - | N/A | - | - |  | - | - | - | - | - | - | 83.8\% |
| Queens <br> Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd | - | - | N/A | - | - |  | - | - | - | - | - | - | 83.8\% |
| 1/2+1/1 | Binstead Rd Entry Left Ahead Right | O+U | N/A | N/A | A B |  | 1 | 51:62 | - | 832 | 1852:1684 | 993 | 83.8\% |
| 2/1 | Queens Rd Entry Right Left Ahead | U | N/A | N/A | C |  | 1 | 16 | - | 309 | 1758 | 374 | 82.7\% |
| 3/1 | Pellhurst Rd Entry Ahead Left | U | N/A | N/A | D |  | 1 | 50 | - | 536 | 1744 | 1112 | 48.2\% |
| 4/1 | Binstead Rd Exit | U | N/A | N/A | - |  | - | - | - | 671 | Inf | Inf | 0.0\% |
| 5/1 | Queens Rd Exit | U | N/A | N/A | - |  | - | - | - | 363 | Inf | Inf | 0.0\% |
| 6/1 | Pellhurst Rd Exit | U | N/A | N/A | - |  | - | - | - | 471 | Inf | Inf | 0.0\% |
| 7/1 | Mayfield Rd Entry Left Ahead Right | U | N/A | N/A | - |  | - | - | - | 0 | Inf | Inf | 0.0\% |
| 8/1 | Mayfield Rd Exit | U | N/A | N/A | - |  | - | - | - | 172 | Inf | Inf | 0.0\% |
| Ped Link: P1 | Unnamed Ped Link | - | N/A | - | F |  | 1 | 52 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P2 | Unnamed Ped Link | - | N/A | - | G |  | 1 | 15 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P3 | Unnamed Ped Link | - | N/A | - | F |  | 1 | 52 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P4 | Unnamed Ped Link | - | N/A | - | H |  | 1 | 6 | - | 0 | - | 0 | 0.0\% |

Full Input Data And Results

| Item | Arriving (pcu) | Leaving (pcu) | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Uniform Delay (pcuHr) | Rand + Oversat Delay (pcuHr) | Storage Area Uniform Delay (pcuHr) | Total Delay (pcuHr) | Av. Delay Per PCU (s/pcu) | Max. Back of Uniform Queue (pcu) | Rand + Oversat Queue (pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: HCA Tender IoW | - | - | 9 | 2 | 0 | 6.4 | 5.2 | 0.0 | 11.6 | - | - | - | - |
| Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd | - | - | 9 | 2 | 0 | 6.4 | 5.2 | 0.0 | 11.6 | - | - | - | - |
| 1/2+1/1 | 832 | 832 | 9 | 2 | 0 | 2.7 | 2.5 | - | 5.2 | 22.6 | 13.7 | 2.5 | 16.2 |
| 2/1 | 309 | 309 | - | - | - | 2.6 | 2.2 | - | 4.8 | 56.2 | 6.5 | 2.2 | 8.8 |
| 3/1 | 536 | 536 | - | - | - | 1.1 | 0.5 | - | 1.6 | 10.7 | 6.1 | 0.5 | 6.6 |
| 4/1 | 671 | 671 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 5/1 | 363 | 363 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 6/1 | 471 | 471 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 7/1 | 0 | 0 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 8/1 | 172 | 172 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Ped Link: P1 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P2 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P3 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P4 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| C1 |  |  | PRC for Signalled Lanes (\%): PRC Over All Lanes (\%): |  | $\begin{aligned} & 7.4 \\ & 7.4 \end{aligned}$ | Total Delay for Signalled Lanes (pcuHr): Total Delay Over All Lanes(pcuHr): |  |  | Cycle Time (s): 80 |  |  |  |  |

Full Input Data And Results
Scenario 6: '2034 PM Base (80 seconds)' (FG4: '2034 - PM - Base', Plan 1: 'Network Control Plan 1') Stage Sequence Diagram


## Stage Timings

| Stage | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
| :---: | :---: | :---: | :---: |
| Duration | 47 | 5 | 8 |
| Change Point | 0 | 54 | 67 |

Signal Timings Diagram



## Full Input Data And Results

## Network Results

| Item | Lane Description | Lane Type | Controller Stream | Position In Filtered Route | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow Green (s) | Demand Flow (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | $\begin{aligned} & \text { Deg Sat } \\ & \text { (\%) } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: HCA Tender loW | - | - | N/A | - | - |  | - | - | - | - | - | - | 110.5\% |
| Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd | - | - | N/A | - | - |  | - | - | - | - | - | - | 110.5\% |
| 1/2+1/1 | Binstead Rd Entry Left Ahead Right | $\mathrm{O}+\mathrm{U}$ | N/A | N/A | A B |  | 1 | 48:62 | - | 1023 | 1844:1684 | 942 | 108.6\% |
| 2/1 | Queens Rd Entry Right Left Ahead | U | N/A | N/A | C |  | 1 | 19 | - | 485 | 1756 | 439 | 110.5\% |
| 3/1 | Pellhurst Rd Entry Ahead Left | U | N/A | N/A | D |  | 1 | 47 | - | 516 | 1793 | 1076 | 48.0\% |
| 4/1 | Binstead Rd Exit | U | N/A | N/A | - |  | - | - | - | 849 | Inf | Inf | 0.0\% |
| 5/1 | Queens Rd Exit | u | N/A | N/A | - |  | - | - | - | 485 | Inf | Inf | 0.0\% |
| 6/1 | $\underset{\text { Exit }}{\text { Pellhurst Rd }}$ | U | N/A | N/A | - |  | - | - | - | 543 | Inf | Inf | 0.0\% |
| 7/1 | Mayfield Rd Entry Left Ahead Right | U | N/A | N/A | - |  | - | - | - | 1 | Inf | Inf | 0.0\% |
| 8/1 | Mayfield Rd Exit | U | N/A | N/A | - |  | - | - | - | 148 | Inf | Inf | 0.0\% |
| Ped Link: P1 | Unnamed Ped Link | - | N/A | - | F |  | 1 | 49 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P2 | Unnamed Ped Link | - | N/A | - | G |  | 1 | 18 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P3 | Unnamed Ped Link | - | N/A | - | F |  | 1 | 49 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P4 | Unnamed Ped Link | - | N/A | - | H |  | 1 | 6 | - | 0 | - | 0 | 0.0\% |

Full Input Data And Results

| Item | Arriving (pcu) | Leaving (pcu) | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Uniform Delay (pcuHr) | Rand + Oversat Delay (pcuHr) | Storage Area Uniform Delay (pcuHr) | Total Delay (pcuHr) | Av. Delay <br> Per PCU <br> (s/pcu) | Max. Back of Uniform Queue (pcu) | Rand + Oversat Queue (pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: HCA Tender loW | - | - | 16 | 3 | 0 | 14.8 | 73.8 | 0.0 | 88.6 | - | - | - | - |
| Queens <br> Rd/Pellhurst <br> Rd/Mayfield <br> Rd/Binstead Rd | - | - | 16 | 3 | 0 | 14.8 | 73.8 | 0.0 | 88.6 | - | - | - | - |
| 1/2+1/1 | 1023 | 942 | 16 | 3 | 0 | 7.3 | 45.9 | - | 53.3 | 187.4 | 23.3 | 45.9 | 69.2 |
| 2/1 | 485 | 439 | - | - | - | 6.2 | 27.4 | - | 33.6 | 249.4 | 12.4 | 27.4 | 39.8 |
| 3/1 | 516 | 516 | - | - | - | 1.3 | 0.5 | - | 1.7 | 12.2 | 6.3 | 0.5 | 6.8 |
| 4/1 | 809 | 809 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 5/1 | 447 | 447 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 6/1 | 500 | 500 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 7/1 | 1 | 1 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 8/1 | 142 | 142 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Ped Link: P1 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P2 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P3 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P4 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| C1 |  |  | PRC for Signalled Lanes (\%): PRC Over All Lanes (\%): |  | $\begin{aligned} & -22.8 \\ & -22.8 \end{aligned}$ | Total Delay for Signalled Lanes (pcuHr): Total Delay Over All Lanes(pcuHr): |  |  | Cycle Time (s): 80 |  | 80 |  |  |

Full Input Data And Results
Full Input Data And Results

## User and Project Details

| Project: | A090129-60 |
| :--- | :--- |
| Title: | HCA Tender IoW |
| Location: |  |
| Additional detail: |  |
| File name: | Junction 8 - Binstead Rd Pellhurst Rd Proposed.Isg3x |
| Author: |  |
| Company: <br> Address: |  |

## Network Layout Diagram



Full Input Data And Results
Phase Input Data

| Phase Name | Phase Type | Assoc. Phase | Street Min | Cont Min |
| :---: | :---: | :---: | :---: | :---: |
| A | Traffic |  | 7 | 7 |
| B | Traffic |  | 7 | 7 |
| C | Traffic |  | 7 | 7 |
| D | Traffic |  | 7 | 7 |
| E | Pedestrian |  | 6 | 6 |
| F | Pedestrian |  | 6 | 6 |
| G | Pedestrian |  | 6 | 6 |

Phase Intergreens Matrix


Phases in Stage

| Stage No. | Phases in Stage |
| :---: | :--- |
| 1 | A B D E |
| 2 | C G |
| 3 | B F |

## Stage Diagram

| 1 | 2 |  |
| :---: | :---: | :---: |
|  |  |  |

## Phase Delays

| Term. Stage | Start Stage | Phase | Type | Value | Cont value |
| :--- | :--- | :--- | :--- | :--- | :--- |

There are no Phase Delays defined

Prohibited Stage Change

|  | To Stage |  |  |  |
| :--- | :--- | :--- | :--- | :--- |
|  |  | 1 | 2 | 3 |
| From | 1 |  | 7 | 8 |
| Stage | 2 | 6 |  | 6 |
|  | 3 | 8 | 8 |  |

## Full Input Data And Results

## Give-Way Lane Input Data

| Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Movement | $\begin{array}{c}\text { Max Flow } \\ \text { when } \\ \text { Giving Way } \\ \text { (PCU/Hr) }\end{array}$ | $\begin{array}{c}\text { Min Flow } \\ \text { when } \\ \text { Giving Way } \\ \text { (PCU/Hr) }\end{array}$ | $\begin{array}{c}\text { Opposing } \\ \text { Lane }\end{array}$ | $\begin{array}{c}\text { Opp. Lane } \\ \text { Coeff. }\end{array}$ | $\begin{array}{c}\text { Opp. } \\ \text { Mvmnts. }\end{array}$ | $\begin{array}{c}\text { Right Turn } \\ \text { Storage (PCU) }\end{array}$ | $\begin{array}{c}\text { Non-Blocking } \\ \text { Storage } \\ \text { (PCU) }\end{array}$ | $\begin{array}{c}\text { RTF }\end{array}$ | $\begin{array}{c}\text { Right Turn } \\ \text { Move up (s) }\end{array}$ |
| in Intergreen |  |  |  |  |  |  |  |  |  |  |
| (PCU) |  |  |  |  |  |  |  |  |  |  |$\}$

Full Input Data And Results
Lane Input Data
Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd

| Lane | Lane Type | Phases | Start Disp. | End Disp. | Physical Length (PCU) | Sat Flow Type | Def User Saturation Flow (PCU/Hr) | Lane Width (m) | Gradient | Nearside Lane | Turns | Turning Radius (m) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{gathered} 1 / 1 \\ \left.\begin{array}{c} \text { (Binstead Rd } \\ \text { Entry) } \end{array}\right) . \end{gathered}$ | U | B | 2 | 3 | 3.0 | Geom | - | 2.80 | 0.00 | Y | Arm 5 Left | 13.00 |
| $1 / 2$ <br> (Binstead Rd Entry) | 0 | A | 2 | 3 | 60.0 | Geom | - | 2.60 | 0.00 | Y | Arm 6 Ahead | Inf |
|  |  |  |  |  |  |  |  |  |  |  | Arm 7 Right | Inf |
| $2 / 1$ <br> (Queens Rd Entry) | U | C | 2 | 3 | 60.0 | Geom | - | 3.20 | 0.00 | Y | Arm 4 Right | 12.00 |
|  |  |  |  |  |  |  |  |  |  |  | Arm 6 Left | 9.00 |
|  |  |  |  |  |  |  |  |  |  |  | Arm 7 <br> Ahead | 12.00 |
| $\begin{aligned} & 3 / 1 \\ & \text { (Pellhurst Rd } \\ & \text { Entry) } \end{aligned}$ | U | D | 2 | 3 | 60.0 | Geom | - | 2.90 | 0.00 | Y | Arm 4 <br> Ahead | Inf |
|  |  |  |  |  |  |  |  |  |  |  | $\text { Arm } 7$ Left | 4.00 |
| $\underset{\substack{\text { (Binstead } \\ \text { Exit) }}}{4 / 1}$ | U |  | 2 | 3 | 60.0 | Inf | - | - | - | - | - | - |
| $\begin{gathered} 5 / 1 \\ \text { (Queens Rd } \\ \text { Exit) } \end{gathered}$ | U |  | 2 | 3 | 60.0 | Inf | - | - | - | - | - | - |
| $\begin{gathered} 6 / 1 \\ \text { (Pellhurst Rd } \\ \text { Exit) } \end{gathered}$ | U |  | 2 | 3 | 60.0 | Inf | - | - | - | - | - | - |
| $\begin{gathered} 7 / 1 \\ \text { (Mayfield } \mathrm{Rd} \\ \text { Exit) } \end{gathered}$ | U |  | 2 | 3 | 60.0 | Inf | - | - | - | - | - | - |

## Traffic Flow Groups

| Flow Group | Start Time | End Time | Duration | Formula |
| :---: | :---: | :---: | :---: | :---: |
| 1: '2034 - AM - DS' | $08: 00$ | $09: 00$ | $01: 00$ |  |
| 2: '2034 - PM - DS' | $17: 00$ | $18: 00$ | $01: 00$ |  |
| 3: '2017 - AM Base' | $08: 00$ | $09: 00$ | $01: 00$ |  |
| 4: '2017 - PM Base' | $17: 00$ | $18: 00$ | $01: 00$ |  |

Full Input Data And Results

Scenario 1: '2017 AM Base' (FG3: '2017 - AM Base', Plan 1: 'Network Control Plan 1') Traffic Flows, Desired Desired Flow :

|  | Destination |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Origin |  | A | B | C | D | Tot. |  |
|  | A | 0 | 297 | 374 | 9 | 680 |  |
|  | B | 218 | 0 | 11 | 24 | 253 |  |
|  | C | 330 | 0 | 0 | 108 | 438 |  |
|  | D | 0 | 0 | 0 | 0 | 0 |  |
|  | Tot. | 548 | 297 | 385 | 141 | 1371 |  |

## Traffic Lane Flows

| Lane | Scenario 1: <br> 2017 AM Base |
| :---: | :---: |
| Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd |  |
| $1 / 1$ <br> (short) | 297 |
| $1 / 2$ <br> (with short) | $680(\mathrm{ln})$ <br> $383($ Out $)$ |
| $2 / 1$ | 253 |
| $3 / 1$ | 438 |
| $4 / 1$ | 548 |
| $5 / 1$ | 297 |
| $6 / 1$ | 385 |
| $7 / 1$ | 141 |

Full Input Data And Results
Lane Saturation Flows

| Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Lane Width (m) | Gradient | Nearside Lane | Allowed Turns | Turning Radius (m) | Turning Prop. | Sat Flow (PCU/Hr) | Flared Sat Flow (PCU/Hr) |
| $1 / 1$ (Binstead Rd Entry) <br> 1/2 <br> (Binstead Rd Entry) | $\begin{aligned} & 2.80 \\ & 2.60 \end{aligned}$ | 0.00 | Y | Arm 5 Left | 13.00 | 100.0 \% | 1699 | 1699 |
|  |  | 0.00 | Y | Arm 6 Ahead | Inf | 97.7 \% | 1875 | 1875 |
|  |  |  |  | Arm 7 Right | Inf | 2.3 \% |  |  |
| $\begin{gathered} 2 / 1 \\ \text { (Queens Rd Entry) } \end{gathered}$ | 3.20 | 0.00 | Y | Arm 4 Right | 12.00 | 86.2 \% | 1717 | 1717 |
|  |  |  |  | Arm 6 Left | 9.00 | 4.3 \% |  |  |
|  |  |  |  | Arm 7 Ahead | 12.00 | 9.5 \% |  |  |
| $3 / 1$ <br> (Pellhurst Rd Entry) | 2.90 | 0.00 | Y | Arm 4 Ahead | Inf | 75.3 \% | 1744 | 1744 |
|  |  |  |  | Arm 7 Left | 4.00 | 24.7 \% |  |  |
| $\begin{gathered} 4 / 1 \\ (\text { Binstead Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 5 / 1 \\ \text { (Queens Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 6 / 1 \\ \text { (Pellhurst Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} \text { 7/1 } \\ \text { (Mayfield Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |

Scenario 2: '2017 PM Base ' (FG4: '2017 - PM Base', Plan 1: 'Network Control Plan 1') Traffic Flows, Desired
Desired Flow :

|  |  | Destination |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Origin |  | A | B | C | D | Tot. |  |
|  | A | 0 | 398 | 425 | 17 | 840 |  |
|  | B | 343 | 0 | 21 | 35 | 399 |  |
|  | C | 353 | 0 | 0 | 71 | 424 |  |
|  | D | 0 | 0 | 0 | 0 | 0 |  |
|  | Tot. | 696 | 398 | 446 | 123 | 1663 |  |

Full Input Data And Results
Traffic Lane Flows

| Lane | Scenario 2: <br> 2017 PM Base |
| :---: | :---: |
| Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd |  |
| $1 / 1$ <br> (short) | 398 |
| $1 / 2$ <br> (with short) | $840(\mathrm{ln})$ <br> $442($ Out) |
| $2 / 1$ | 399 |
| $3 / 1$ | 424 |
| $4 / 1$ | 696 |
| $5 / 1$ | 398 |
| $6 / 1$ | 446 |
| $7 / 1$ | 123 |

## Lane Saturation Flows

| Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Lane Width (m) | Gradient | Nearside Lane | Allowed Turns | Turning Radius (m) | Turning Prop. | Sat Flow (PCU/Hr) | Flared Sat Flow (PCU/Hr) |
| $1 / 1$ (Binstead Rd Entry) | 2.80 | 0.00 | Y | Arm 5 Left | 13.00 | 100.0 \% | 1699 | 1699 |
| $1 / 2$(Binstead Rd Entry) | 2.60 | 0.00 | Y | Arm 6 Ahead | Inf | 96.2 \% | 1875 | 1875 |
|  |  |  |  | Arm 7 Right | Inf | 3.8 \% |  |  |
| $\begin{gathered} \text { 2/1 } \\ \text { (Queens Rd Entry) } \end{gathered}$ | 3.20 | 0.00 | Y | Arm 4 Right | 12.00 | 86.0 \% | 1717 | 1717 |
|  |  |  |  | Arm 6 Left | 9.00 | 5.3 \% |  |  |
|  |  |  |  | Arm 7 Ahead | 12.00 | 8.8 \% |  |  |
| $\begin{gathered} 3 / 1 \\ \text { (Pellhurst Rd Entry) } \end{gathered}$ | 2.90 | 0.00 | Y | Arm 4 Ahead | Inf | 83.3 \% | 1792 | 1792 |
|  |  |  |  | Arm 7 Left | 4.00 | 16.7 \% |  |  |
| $4 / 1$ (Binstead Rd Exit Lane 1) | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 5 / 1 \\ \text { (Queens Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{array}{\|c} 6 / 1 \\ \text { (Pellhurst Rd Exit Lane 1) } \end{array}$ | Infinite Saturation FlowInfinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| 7/1 (Mayfield Rd Exit Lane 1) |  |  |  |  |  |  | Inf | Inf |

Full Input Data And Results
Scenario 3: '2034 - AM - DS' (FG1: '2034 - AM - DS', Plan 1: 'Network Control Plan 1')
Traffic Flows, Desired
Desired Flow :

|  |  | Destination |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Origin |  | A | B | C | D | Tot. |  |
|  | A | 0 | 363 | 458 | 11 | 832 |  |
|  | C | 404 | 0 | 0 | 132 | 536 |  |
|  | D | 0 | 0 | 0 | 0 | 0 |  |
|  | Tot. | 671 | 363 | 471 | 172 | 1677 |  |

## Traffic Lane Flows

| Lane | Scenario 3: <br> 2034 - AM - DS |
| :---: | :---: |
| Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd |  |$|$| $1 / 1$ |  |
| :---: | :---: |
| (short) | 363 |
| $1 / 2$ <br> (with short) <br> $2 / 1$ | $832(\mathrm{ln})$ <br> $469($ Out ) |
| $3 / 1$ | 309 |
| $4 / 1$ | 536 |
| $5 / 1$ | 671 |
| $6 / 1$ | 363 |
| $7 / 1$ | 471 |

## Lane Saturation Flows

| Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Lane Width (m) | Gradient | Nearside Lane | Allowed Turns | Turning Radius (m) | Turning Prop. | Sat Flow (PCU/Hr) | Flared Sat Flow (PCU/Hr) |
| $1 / 1$ (Binstead Rd Entry) | 2.80 | 0.00 | Y | Arm 5 Left | 13.00 | 100.0 \% | 1699 | 1699 |
| $1 / 2$(Binstead Rd Entry) | 2.60 | 0.00 | Y | Arm 6 Ahead | Inf | 97.7\% | 1875 | 1875 |
|  |  |  |  | Arm 7 Right | Inf | 2.3 \% |  |  |
| $\begin{gathered} 2 / 1 \\ \text { (Queens Rd Entry) } \end{gathered}$ | 3.20 | 0.00 | Y | Arm 4 Right | 12.00 | 86.4 \% | 1717 | 1717 |
|  |  |  |  | Arm 6 Left | 9.00 | 4.2 \% |  |  |
|  |  |  |  | Arm 7 Ahead | 12.00 | 9.4 \% |  |  |
| $\begin{gathered} 3 / 1 \\ \text { (Pellhurst Rd Entry) } \end{gathered}$ | 2.90 | 0.00 | Y | Arm 4 Ahead | Inf | 75.4 \% | 1744 | 1744 |
|  |  |  |  | Arm 7 Left | 4.00 | 24.6 \% |  |  |
| $\begin{gathered} 4 / 1 \\ \text { (Binstead Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 5 / 1 \\ \text { (Queens Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 6 / 1 \\ \text { (Pellhurst Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 7 / 1 \\ \text { (Mayfield Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |

Full Input Data And Results

Scenario 4: '2034 - PM - DS' (FG2: '2034 - PM - DS', Plan 1: 'Network Control Plan 1')
Traffic Flows, Desired
Desired Flow :

|  | Destination |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Origin |  | A | B | C | D | Tot. |  |
|  | A | 0 | 485 | 518 | 20 | 1023 |  |
|  | B | 418 | 0 | 25 | 42 | 485 |  |
|  | C | 430 | 0 | 0 | 86 | 516 |  |
|  | D | 0 | 0 | 0 | 0 | 0 |  |
|  | Tot. | 848 | 485 | 543 | 148 | 2024 |  |

## Traffic Lane Flows

| Lane | Scenario 4: <br> 2034 - PM - DS |
| :---: | :---: |
| Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd |  |
| $1 / 1$ <br> (short) <br> 1/2 <br> (with short) | 485 |
| $2 / 1$ | $1023($ In $)$ <br> $538(O u t)$ |
| $3 / 1$ | 485 |
| $4 / 1$ | 516 |
| $5 / 1$ | 848 |
| $6 / 1$ | 485 |
| $7 / 1$ | 543 |

Full Input Data And Results
Lane Saturation Flows

| Junction: Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Lane Width (m) | Gradient | Nearside Lane | Allowed Turns | Turning Radius (m) | Turning Prop. | Sat Flow (PCU/Hr) | Flared Sat Flow (PCU/Hr) |
| $1 / 1$(Binstead Rd Entry)$1 / 2$(Binstead Rd Entry) | $\begin{aligned} & 2.80 \\ & 2.60 \end{aligned}$ | 0.00 | Y | Arm 5 Left <br> Arm 6 Ahead | 13.00 | 100.0 \% | 1699 | 1699 |
|  |  | 0.00 | Y |  | Inf | 96.3 \% | 1875 | 1875 |
|  |  |  |  | Arm 7 Right | Inf | 3.7 \% |  |  |
| $\begin{gathered} 2 / 1 \\ \text { (Queens Rd Entry) } \end{gathered}$ | 3.20 | 0.00 | Y | Arm 4 Right | 12.00 | 86.2 \% | 1717 | 1717 |
|  |  |  |  | Arm 6 Left | 9.00 | 5.2 \% |  |  |
|  |  |  |  | Arm 7 Ahead | 12.00 | 8.7 \% |  |  |
| $3 / 1$ <br> (Pellhurst Rd Entry) | 2.90 | 0.00 | Y | Arm 4 Ahead | Inf | 83.3 \% | 1793 | 1793 |
|  |  |  |  | Arm 7 Left | 4.00 | 16.7 \% |  |  |
| $4 / 1$ (Binstead Rd Exit Lane 1) | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 5 / 1 \\ \text { (Queens Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $\begin{gathered} 6 / 1 \\ \text { (Pellhurst Rd Exit Lane 1) } \end{gathered}$ | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |
| $7 / 1$ (Mayfield Rd Exit Lane 1) | Infinite Saturation Flow |  |  |  |  |  | Inf | Inf |

Scenario 1: '2017 AM Base' (FG3: '2017 - AM Base', Plan 1: 'Network Control Plan 1') Stage Sequence Diagram


## Stage Timings

| Stage | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
| :---: | :---: | :---: | :---: |
| Duration | 21 | 7 | 6 |
| Change Point | 0 | 29 | 43 |

Full Input Data And Results
Signal Timings Diagram



## Network Results

| Item | Lane Description | Lane Type | Controller Stream | Position In Filtered Route | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow Green (s) | Demand Flow (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | Deg Sat <br> (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: HCA Tender IoW | - | - | N/A | - | - |  | - | - | - | - | - | - | 119.8\% |
| Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd | - | - | N/A | - | - |  | - | - | - | - | - | - | 119.8\% |
| 1/2+1/1 | Binstead Rd Entry Left Ahead Right | O+U | N/A | N/A | A B |  | 1 | 21:35 | - | 680 | 1875:1699 | 568 | 119.8\% |
| 2/1 | Queens Rd Entry Right Left Ahead | U | N/A | N/A | C |  | 1 | 7 | - | 253 | 1717 | 250 | 101.3\% |
| 3/1 | Pellhurst Rd Entry Ahead Left | U | N/A | N/A | D |  | 1 | 21 | - | 438 | 1744 | 698 | 62.8\% |
| 4/1 | Binstead Rd Exit | U | N/A | N/A | - |  | - | - | - | 548 | Inf | Inf | 0.0\% |
| 5/1 | Queens Rd Exit | U | N/A | N/A | - |  | - | - | - | 297 | Inf | Inf | 0.0\% |
| 6/1 | $\begin{aligned} & \text { Pellhurst Rd } \\ & \text { Exit } \end{aligned}$ | U | N/A | N/A | - |  | - | - | - | 385 | Inf | Inf | 0.0\% |
| 7/1 | Maytield Rd Exit | U | N/A | N/A | - |  | - | - | - | 141 | Inf | Inf | 0.0\% |
| Ped Link: P1 | Unnamed Ped Link | - | N/A | - | F |  | 1 | 6 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P2 | Unnamed Ped Link | - | N/A | - | E |  | 1 | 29 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P3 | Unnamed Ped Link | - | N/A | - | G |  | 1 | 7 | - | 0 | - | 0 | 0.0\% |

Full Input Data And Results

| Item | Arriving (pcu) | Leaving (pcu) | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Uniform Delay (pcuHr) | Rand + Oversat Delay (pcuHr) | Storage Area Uniform Delay (pcuHr) | Total Delay (pcuHr) | Av. Delay <br> Per PCU <br> (s/pcu) | Max. Back of Uniform Queue (pcu) | Rand + Oversat Queue (pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: HCA Tender IoW | - | - | 6 | 2 | 0 | 8.6 | 68.8 | 0.0 | 77.3 | - | - | - | - |
| Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd | - | - | 6 | 2 | 0 | 8.6 | 68.8 | 0.0 | 77.3 | - | - | - | - |
| 1/2+1/1 | 680 | 568 | 6 | 2 | 0 | 5.2 | 59.1 | - | 64.3 | 340.7 | 10.3 | 59.1 | 69.5 |
| 2/1 | 253 | 250 | - | - | - | 1.7 | 8.8 | - | 10.5 | 149.7 | 3.9 | 8.8 | 12.7 |
| 3/1 | 438 | 438 | - | - | - | 1.6 | 0.8 | - | 2.4 | 20.1 | 5.4 | 0.8 | 6.2 |
| 4/1 | 545 | 545 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 5/1 | 248 | 248 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 6/1 | 323 | 323 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 7/1 | 139 | 139 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Ped Link: P1 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P2 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P3 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| C1 |  |  | PRC for Signalled Lanes (\%): PRC Over All Lanes (\%): |  | $\begin{aligned} & -33.1 \\ & -33.1 \end{aligned}$ | Total Delay for Signalled Lanes (pcuHr): Total Delay Over All Lanes(pcuHr): |  |  | Cycle Time (s): 55 |  | 55 |  |  |

Full Input Data And Results
Scenario 2: '2017 PM Base ' (FG4: '2017 - PM Base', Plan 1: 'Network Control Plan 1')
Stage Sequence Diagram


## Stage Timings

| Stage | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
| :---: | :---: | :---: | :---: |
| Duration | 20 | 8 | 6 |
| Change Point | 0 | 28 | 43 |

Signal Timings Diagram



## Full Input Data And Results

## Network Results

| Item | Lane Description | Lane Type | Controller Stream | Position In Filtered Route | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow Green (s) | Demand Flow (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | Deg Sat <br> (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: HCA Tender IoW | - | - | N/A | - | - |  | - | - | - | - | - | - | 142.0\% |
| Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd | - | - | N/A | - | - |  | - | - | - | - | - | - | 142.0\% |
| 1/2+1/1 | Binstead Rd Entry Left Ahead Right | O+U | N/A | N/A | A B |  | 1 | 20:34 | - | 840 | 1875:1699 | 600 | 139.9\% |
| 2/1 | Queens Rd Entry Right Left Ahead | U | N/A | N/A | C |  | 1 | 8 | - | 399 | 1717 | 281 | 142.0\% |
| 3/1 | Pellhurst Rd Entry Ahead Left | U | N/A | N/A | D |  | 1 | 20 | - | 424 | 1792 | 684 | 62.0\% |
| 4/1 | Binstead Rd Exit | U | N/A | N/A | - |  | - | - | - | 696 | Inf | Inf | 0.0\% |
| 5/1 | Queens Rd Exit | U | N/A | N/A | - |  | - | - | - | 398 | Inf | Inf | 0.0\% |
| 6/1 | $\begin{aligned} & \text { Pellhurst Rd } \\ & \text { Exit } \end{aligned}$ | U | N/A | N/A | - |  | - | - | - | 446 | Inf | Inf | 0.0\% |
| 7/1 | Maytield Rd Exit | U | N/A | N/A | - |  | - | - | - | 123 | Inf | Inf | 0.0\% |
| Ped Link: P1 | Unnamed Ped Link | - | N/A | - | F |  | 1 | 6 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P2 | Unnamed Ped Link | - | N/A | - | E |  | 1 | 28 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P3 | Unnamed Ped Link | - | N/A | - | G |  | 1 | 8 | - | 0 | - | 0 | 0.0\% |

Full Input Data And Results

| Item | Arriving (pcu) | Leaving (pcu) | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Uniform Delay (pcuHr) | Rand + Oversat Delay (pcuHr) | Storage Area Uniform Delay (pcuHr) | Total Delay (pcuHr) | Av. Delay <br> Per PCU <br> (s/pcu) | Max. Back of Uniform Queue (pcu) | Rand + Oversat Queue (pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: HCA Tender IoW | - | - | 10 | 3 | 0 | 16.4 | 183.1 | 0.0 | 199.5 | - | - | - | - |
| Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd | - | - | 10 | 3 | 0 | 16.4 | 183.1 | 0.0 | 199.5 | - | - | - | - |
| 1/2+1/1 | 840 | 600 | 10 | 3 | 0 | 9.2 | 121.6 | - | 130.8 | 560.4 | 14.5 | 121.6 | 136.1 |
| 2/1 | 399 | 281 | - | - | - | 5.6 | 60.7 | - | 66.3 | 598.0 | 8.7 | 60.7 | 69.4 |
| 3/1 | 424 | 424 | - | - | - | 1.6 | 0.8 | - | 2.4 | 20.6 | 5.2 | 0.8 | 6.0 |
| 4/1 | 595 | 595 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 5/1 | 284 | 284 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 6/1 | 318 | 318 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 7/1 | 108 | 108 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Ped Link: P1 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P2 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P3 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| C1 |  |  | PRC for Signalled Lanes (\%): PRC Over All Lanes (\%): |  | $\begin{aligned} & -57.8 \\ & -57.8 \end{aligned}$ | Total Delay for Signalled Lanes (pcuHr): Total Delay Over All Lanes(pcuHr): |  |  | Cycle Time (s): $\quad 55$ |  |  |  |  |

Full Input Data And Results
Scenario 3: '2034-AM - DS' (FG1: '2034 - AM - DS', Plan 1: 'Network Control Plan 1')
Stage Sequence Diagram


## Stage Timings

| Stage | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
| :---: | :---: | :---: | :---: |
| Duration | 21 | 7 | 6 |
| Change Point | 0 | 29 | 43 |

Signal Timings Diagram



## Full Input Data And Results

## Network Results

| Item | Lane Description | Lane Type | Controller Stream | Position In Filtered Route | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow Green (s) | Demand Flow (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | Deg Sat <br> (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: HCA Tender IoW | - | - | N/A | - | - |  | - | - | - | - | - | - | 195.6\% |
| Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd | - | - | N/A | - | - |  | - | - | - | - | - | - | 195.6\% |
| 1/2+1/1 | Binstead Rd Entry Left Ahead Right | O+U | N/A | N/A | A B |  | 1 | 21:35 | - | 832 | 1875:1699 | 425 | 195.6\% |
| 2/1 | Queens Rd Entry Right Left Ahead | U | N/A | N/A | C |  | 1 | 7 | - | 309 | 1717 | 250 | 123.7\% |
| 3/1 | Pellhurst Rd Entry Ahead Left | U | N/A | N/A | D |  | 1 | 21 | - | 536 | 1744 | 698 | 76.8\% |
| 4/1 | Binstead Rd Exit | U | N/A | N/A | - |  | - | - | - | 671 | Inf | Inf | 0.0\% |
| 5/1 | Queens Rd Exit | U | N/A | N/A | - |  | - | - | - | 363 | Inf | Inf | 0.0\% |
| 6/1 | $\begin{aligned} & \text { Pellhurst Rd } \\ & \text { Exit } \end{aligned}$ | U | N/A | N/A | - |  | - | - | - | 471 | Inf | Inf | 0.0\% |
| 7/1 | Maytield Rd Exit | U | N/A | N/A | - |  | - | - | - | 172 | Inf | Inf | 0.0\% |
| Ped Link: P1 | Unnamed Ped Link | - | N/A | - | F |  | 1 | 6 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P2 | Unnamed Ped Link | - | N/A | - | E |  | 1 | 29 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P3 | Unnamed Ped Link | - | N/A | - | G |  | 1 | 7 | - | 0 | - | 0 | 0.0\% |

Full Input Data And Results

| Item | Arriving (pcu) | Leaving (pcu) | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Uniform Delay (pcuHr) | Rand + Oversat Delay (pcuHr) | Storage Area Uniform Delay (pcuHr) | Total Delay (pcuHr) | Av. Delay <br> Per PCU <br> (s/pcu) | Max. Back of Uniform Queue (pcu) | Rand + Oversat Queue (pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: HCA Tender IoW | - | - | 4 | 2 | 0 | 18.6 | 238.0 | 0.0 | 256.6 | - | - | - | - |
| Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd | - | - | 4 | 2 | 0 | 18.6 | 238.0 | 0.0 | 256.6 | - | - | - | - |
| 1/2+1/1 | 832 | 425 | 4 | 2 | 0 | 13.1 | 204.4 | - | 217.5 | 941.2 | 17.5 | 204.4 | 221.9 |
| 2/1 | 309 | 250 | - | - | - | 3.3 | 32.0 | - | 35.3 | 411.4 | 5.8 | 32.0 | 37.8 |
| 3/1 | 536 | 536 | - | - | - | 2.1 | 1.6 | - | 3.8 | 25.2 | 7.0 | 1.6 | 8.6 |
| 4/1 | 620 | 620 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 5/1 | 186 | 186 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 6/1 | 245 | 245 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 7/1 | 161 | 161 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Ped Link: P1 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P2 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P3 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| C1 |  |  | PRC for Signalled Lanes (\%): PRC Over All Lanes (\%): |  | $\begin{aligned} & -117.4 \\ & -117.4 \end{aligned}$ | Total Delay for Signalled Lanes (pcuHr): Total Delay Over All Lanes(pcuHr): |  |  | Cycle Time (s): 55 |  |  |  |  |

Full Input Data And Results
Scenario 4: '2034-PM - DS' (FG2: '2034 - PM - DS', Plan 1: 'Network Control Plan 1')
Stage Sequence Diagram


## Stage Timings

| Stage | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
| :---: | :---: | :---: | :---: |
| Duration | 21 | 7 | 6 |
| Change Point | 0 | 29 | 43 |

Signal Timings Diagram



## Full Input Data And Results

## Network Results

| Item | Lane Description | Lane Type | Controller Stream | Position In Filtered Route | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow Green (s) | Demand Flow (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | Deg Sat <br> (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: HCA Tender IoW | - | - | N/A | - | - |  | - | - | - | - | - | - | 194.2\% |
| Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd | - | - | N/A | - | - |  | - | - | - | - | - | - | 194.2\% |
| 1/2+1/1 | Binstead Rd Entry Left Ahead Right | O+U | N/A | N/A | A B |  | 1 | 21:35 | - | 1023 | 1875:1699 | 536 | 191.0\% |
| 2/1 | Queens Rd Entry Right Left Ahead | U | N/A | N/A | C |  | 1 | 7 | - | 485 | 1717 | 250 | 194.2\% |
| 3/1 | Pellhurst Rd Entry Ahead Left | U | N/A | N/A | D |  | 1 | 21 | - | 516 | 1793 | 717 | 71.9\% |
| 4/1 | Binstead Rd Exit | U | N/A | N/A | - |  | - | - | - | 848 | Inf | Inf | 0.0\% |
| 5/1 | Queens Rd Exit | U | N/A | N/A | - |  | - | - | - | 485 | Inf | Inf | 0.0\% |
| 6/1 | $\begin{aligned} & \text { Pellhurst Rd } \\ & \text { Exit } \end{aligned}$ | U | N/A | N/A | - |  | - | - | - | 543 | Inf | Inf | 0.0\% |
| 7/1 | Maytield Rd Exit | U | N/A | N/A | - |  | - | - | - | 148 | Inf | Inf | 0.0\% |
| Ped Link: P1 | Unnamed Ped Link | - | N/A | - | F |  | 1 | 6 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P2 | Unnamed Ped Link | - | N/A | - | E |  | 1 | 29 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P3 | Unnamed Ped Link | - | N/A | - | G |  | 1 | 7 | - | 0 | - | 0 | 0.0\% |

Full Input Data And Results

| Item | Arriving (pcu) | Leaving (pcu) | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Uniform Delay (pcuHr) | Rand + Oversat Delay (pcuHr) | Storage Area Uniform Delay (pcuHr) | Total Delay (pcuHr) | Av. Delay <br> Per PCU <br> (s/pcu) | Max. Back of Uniform Queue (pcu) | Rand + Oversat Queue (pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: HCA Tender IoW | - | - | 8 | 3 | 0 | 28.0 | 364.7 | 0.0 | 392.7 | - | - | - | - |
| Queens Rd/Pellhurst Rd/Mayfield Rd/Binstead Rd | - | - | 8 | 3 | 0 | 28.0 | 364.7 | 0.0 | 392.7 | - | - | - | - |
| 1/2+1/1 | 1023 | 536 | 8 | 3 | 0 | 15.9 | 244.8 | - | 260.7 | 917.5 | 21.2 | 244.8 | 266.0 |
| 2/1 | 485 | 250 | - | - | - | 10.0 | 118.6 | - | 128.7 | 955.1 | 13.4 | 118.6 | 132.0 |
| 3/1 | 516 | 516 | - | - | - | 2.0 | 1.3 | - | 3.3 | 22.7 | 6.6 | 1.3 | 7.9 |
| 4/1 | 645 | 645 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 5/1 | 254 | 254 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 6/1 | 284 | 284 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 7/1 | 118 | 118 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Ped Link: P1 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P2 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| Ped Link: P3 | 0 | 0 | - | - | - | - | - | - | - | - | - | - | - |
| C1 |  |  | PRC for Signalled Lanes (\%): PRC Over All Lanes (\%): |  | $\begin{aligned} & -115.8 \\ & -115.8 \end{aligned}$ | Total Delay for Signalled Lanes (pcuHr): Total Delay Over All Lanes(pcuHr): |  |  | Cycle Time (s): 55 |  |  |  |  |

